

Masterplan Drainage Strategy

Walshaw Garden Neighbourhood, Bury

for

HIMOR, Redrow Homes and VHW Land Partnership (Walshaw) Limited



20/10/2020



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1.0 Introduction

1.1 Purpose of This Report

This masterplan drainage strategy report has been developed to support the allocation of the site through the GMSF associated with application for the development of a circa 64-hectare comprising mainly greenfield site referred to as the Walshaw Garden Neighbourhood for the provision of up to 1,250 residential dwellings and a one form entry primary school development; associated landscaping, roads and related works on land northwest of Bury town centre, east of Walshaw.

Specifically, this drainage strategy report relates to the outline masterplan drainage strategy associated with the proposed residential developments and single form entry school to be brought forward independently by the separate four landownerships comprising HIMOR, Redrow Homes, VHW Land Partnership (Walshaw) Limited and Bury Council.

It presents the methodology and calculations associated with the latest current outline masterplan drainage proposals with potential future constraints identified during the development of the drainage strategy.

1.2 Proposed Development

The current proposals for the Walshaw Garden Neighbourhood Development Framework November 2019 prepared by Barton Wilmore for the development of the site includes for up to 1,250 homes; a one form entry primary school (use class D1), associated landscaping, roads and related works.

A Site Location and Land Ownership Plan is contained within Appendix A and the current Concept Masterplan developed by Barton Wilmore is contained in Appendix B.

1.3 RoC Flood Risk Assessments

RoC Consulting (RoC) have developed the current Flood Risk Assessments for the three sites under the ownership HIMOR, Redrow Homes and VHW Land Partnership (Walshaw) Limited, which incorporates the Bury Council site:

- 4072 / HIMOR FRA
- 4072 / Redrow Homes FRA
- 4072 / VHW & BC FRA



2.0 Policy & Guidance

2.1 Bury Unitary Development Plan

The Bury Unitary Development Plan (UDP) is a guide for the future development or protection of land in the Borough and the policies and proposals currently form the basis of the Council's decision on planning applications.

The current Bury UDP was adopted by the Council on the 29th August 1997. The Council is now working to replace the adopted UDP with a new document called the Bury Local Plan. Until the new Local Plan is produced the UDP will continue to be used to make planning decisions.

In relation to drainage and flood risk, the following policies are relevant from the UDP Part 2 – Chapter 6: Environment and are consistent with the current National Planning Policy Framework and Guidelines (NPPF & NPPG).

EN5 - FLOOD PROTECTION AND DEFENCE

The Council will seek to control development in a manner consistent with flood protection and the maintenance of flood defence systems.

EN5/1 - NEW DEVELOPMENT AND FLOOD RISK

The Council will not permit new development, including the raising of land and the intensification of development, where such development would be at risk from flooding, would be likely to increase the risk of flooding elsewhere, or would adversely affect river flood defences. When assessing proposals against this policy, the following criteria will apply:

- the impact on the floodplain
- the increase in surface water run-off as a result of the proposal
- the impact on fluvial flood defences
- the incorporation of mitigating works

EN5	FLOOD PROTECTION AND DEFENCE	4	7, 17 (Bullet 6), 100	Policy ENS seeks to guide development away from areas that may be at risk from flooding and to restrict development that would itself increase the risk of flooding. This is in conformity with the NPPF which states that inappropriate development in areas at risk of flooding should be avoided by directing development away from areas at highest risk.
	EN5/1 – New Development and Flood Risk	4	7, 17 (Bullet 6), 100	Policy EN5/1 seeks to ensure that new development or the intensification of existing development is not at risk from flooding and does not increase the risk of flooding elsewhere. This is in conformity with the NPPF which states that inappropriate development in areas at risk of flooding should be avoided by directing development away from areas at highest risk.



EN7 - POLLUTION CONTROL

The Council will seek to control environmental nuisance and minimise pollution levels associated with development by limiting the environmental impact of pollution, wherever possible, in conformity with current legislation and prescribed standards.

EN7 POLI	LLUTION CONTROL		7, 17, 109, 110, 120, 121, 122, 124	Policy EN7 states that the Council will seek to control environmental nuisance and minimise pollution levels associated with development by limiting the environmental impact of pollution. This is considered to be consistent with the NPPF in that pollution control is one of the integral aspects of securing sustainable development and is one of the core planning principles. It is also consistent with the approach to controlling pollution as set out in section 11 of the NPPF.
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EN7/3 - WATER POLLUTION

The Council will not permit development which would have an unacceptable adverse effect in terms of pollution upon the water quality of the Borough's water courses and other water features.

EN7/4 - GROUNDWATER PROTECTION

The Council will not permit development proposals which would have an unacceptable adverse effect on groundwater resources, particularly in terms of their quality and/or supply.

EN7/3 – Water Pollution	~	7, 17, 109, 110, 120, 121, 122, 124	Policy EN7/3 states that the Council will not permit development which would have an unacceptable adverse effect in terms of pollution upon the water quality of the Borough's water courses and other water features. This is considered to be consistent with the NPPF in that pollution control is one of the integral aspects of securing sustainable development and is one of the core planning principles. It is also consistent with the approach to controlling pollution as set out in section 11 of the NPPF.
EN7/4 – Groundwater Protection	~	7, 17, 109, 110, 120, 121, 122, 124	Policy EN7/4 states that the Council will not permit development proposals which would have an unacceptable adverse effect on groundwater resources, particularly in terms of their quality and/or supply. This is considered to be consistent with the NPPF in that pollution control is one of the integral aspects of securing sustainable development and is one of the core planning principles. It is also consistent with the approach to controlling pollution as set out in section 11 of the NPPF.

2.2 C753 SUDS Manual

This document provides best practice guidance on the planning, design, construction, operation and maintenance of sustainable drainage systems (SUDS). This document provides detail on all the typical sustainable drainage systems and details on how they can be interconnected to not only provide the required drainage performance but also act as pollution control whilst enhancing the site wide masterplan proposals.



SUDS should not be thought of as an individual component, but as an interconnected system designed to manage, treat and make best use of surface water, from where it falls as rain to the point at which it is discharged into the receiving environment beyond the boundaries of the site.

A central design concept for SUDS is the SUDS Management Train. This describes the use of a sequence of components that collectively provides the necessary processes to control the frequency of run-off, the flow rates and the volumes of run-off, and to reduce concentrations of contaminants to acceptable levels. There are six specific functions provided by SUDS components. They are not independent, and one component may provide two or more functions.

2.3 Non-Statutory Technical Standards for SUDS

This document sets out the non-statutory technical standards for sustainable drainage systems. They should be used in conjunction with the National Planning Policy Framework and Planning Practice Guidance.

The technical standards prescribe requirements for both Peak Flow and Volume Control with an emphasis on ensuring that these do not exceed the pre-development case and, where practicable, should not exceed greenfield rates and volumes for the 1 in 1 year and 1 in 100 year rainfall events.

Previous undeveloped site peak flow rates and run-off volumes must never exceed existing greenfield figures.

A practice guidance document was produced by the LASOO advisory group in support of the Non-Statutory technical standards. In August 2019, LASOO was replaced by the Association of SUDS Authorities (ASA), the 'standards' currently remain.



3.0 Site Context

The development site is located to the north and south of Walshaw Road in the Walshaw area of Bury, approximately 2.5km to the northwest of the town centre.

The masterplan development site is centred around NGR: SD 78058 11680 with an approximate nearest postcode of BL8 3AE. The total masterplan red line site application boundary is 64 hectares including 2.90 hectares within the site red line application boundary for the Bury Council land. The site is roughly split as Residential and School for 37.5 hectares with 26.5 hectares for green infrastructure.

A Barton Willmore Site Location and Land Ownership Plan is contained within Appendix A.

As noted above, the masterplan development site is split between four landownerships and three independent Flood Risk Assessments have been produced for each part of the site for each landowner, based on the Barton Willmore Concept Masterplan contained within Appendix B.

The proposed development comprises the construction of circa 1,250 residential units spread across an approximate total development area with an assumed density of approximately 34.6 dwellings per hectare (dph).

For the purposes of this strategy and based upon the areas provided by Barton Willmore, it has been assumed that 60% of the net development area will be impermeable (roof, paved areas, roads and footpaths etc) including a 10% allowance for urban creep providing the positively drained area. The proposed use of SUDS techniques is to be further developed at detailed design stage and to meet any reserved planning matters that will allow for flexibility in the percentage.

For a detailed description of each part of the site refer to the individual Flood Risk Assessments.

In general terms, the site is bounded to the north by Scobell Street. Bisected west east by Walshaw Road roughly through the centre of the site. Dow Lane and greenfield to the south. Mainly residential with Church Street, High Street and Lowercroft Road to the west.

The majority of the site is currently undeveloped greenfield land in the form of fields separated by hedgerows. The only areas of hard standing are relatively small (possibly temporary overspill car parking) associated with the Stables Country Club.

A review of the existing site levels indicates that for the northern development site area from Scobell Street to the north; to a high point adjacent to Christ Church, Walshaw; falls south and north respectively towards Walshaw Brook. The southern development area from the high point at Christ Church, Walshaw; falls south easterly towards Elton Brook. Refer to RoC drawings



4072/SK104, 105 and 106 located in Appendix C for plans indicating existing site contours and overland flow routes.

There are three notable watercourses either within or in close proximity to the site:-

- 1. To the north of Scobell Street, in close proximity to the northern boundary of the site an unnamed watercourse is present which flows through the existing residential area from west to east before becoming culverted approximately 50m to the west of Camberley Close. United Utilities records identify the culvert turning and flowing in a north easterly direction. There is no evidence to suggest that it enters the proposed development site.
- 2. Walshaw Brook is an ordinary watercourse which runs through the northern part of the site to the southern boundaries of the HIMOR, Redrow Homes land and the northern boundary of the VHW Land Partnership (Walshaw) Limited land, flowing in a south easterly direction. There are numerous tributaries that drain at various locations along its length. Also, there are ponds/lakes to the north of the Brook and to the south of the Redrow Homes site which have connectivity.
- 3. Elton Brook is also an ordinary watercourse present just outside the VHW Land Partnership (Walshaw) Limited's southern development site boundary. This generally flows in an easterly direction with a number of tributaries present within the site connecting at various points. One of the main tributaries commences within the site and crosses a number of the development parcels. Outside the south east boundary, north of Elton Vale sports Club, are three large water features which are referred to as Elton the Brook Reservoirs.

United Utilities are the local water company, and their sewer record maps show that the highways surrounding the development site are well served by an existing network of sewers. A combined public sewer doglegs across the VHW Land Partnership (Walshaw) Limited from Sudren Street in a south easterly direction, to the east of Dow Lane. These are located in Appendix D

There are no records of private sewers within the development site boundary.

For details on the masterplan development sites geology, hydrogeology, ground water protection zones, historical flood records, flood mapping etc refer to the RoC site specific Flood Risk Assessments.



4.0 Foul Water Drainage

4.1 Foul Water Drainage Strategy

The general principle of the foul drainage strategy is to provide new separate foul drainage systems with connections to the exiting public sewer network surrounding the site at points to suit the individual development site proposals.

Given the varying level of the development site and requirement for three ownership sites to be independently drained, it is envisaged that one or more pumping stations may be required which would be offered to United Utilities for adoption.

Preliminary calculations have been undertaken based on the Sewers for Adoption allowance of 4,000 litres per dwelling per day and based on the current predicted level of development proposed of 1,250 properties. The estimated design flow for the masterplan development site would be in the order of 61.6 l/s, including an allowance of 3.7l/s for the one form entry primary school. However, this will be subject to review as the scheme is developed and therefore the proposed foul water system strategy has been developed with some flexibility to allow for the phased nature of the scheme and the potential for the masterplan to evolve over time.

The table below summarises the split of the properties based on the masterplan development site net developable area of 36.12ha, based on 34.6 dwellings/ha and the estimated total design foul flow rate.

Land Parcel	Area (ha)	Property No.	Flow Rate (I/s)
Masterplan			
Development	36.12	1250	61.6
Site			

4.1.1 HIMOR

The masterplan foul water drainage strategy drawing 4072/SK111 can be located in Appendix I, highlighting the indicative location of the gravity foul water drainage, a pumping station and pumping main are based on the current Barton Willmore Concept Masterplan (Refer to Appendix B), together with outfall points to the United Utilities combined sewerage network.

Preliminary calculations have been undertaken based on the Sewers for Adoption allowance of 4,000 litres per dwelling per day and based on the current predicted level of development proposed of 261 properties. The estimated design flow for the HIMOR development site would be in the order of 12.1 l/s. However, this will be subject to review as the scheme is developed and therefore



the proposed foul water system strategy has been developed with some flexibility to allow for the phased nature of the scheme and the potential for the masterplan to evolve over time.

The table below summarises the split of the properties based on the individual development site areas based on 34.6 dwellings/ha, and the estimated design foul flow rates.

Land Parcel	Area (ha)	Property No.	Flow Rate (I/s)
HIMOR	7.54	261	12.1

Based on the existing concept masterplan layout, development plot catchment plan (Refer to appendix E), existing site contour levels (Refer to Appendix C), the location and levels of the combined public sewerage network from the sewer record maps (Refer to Appendix D), it is currently assumed that an adoptable pumping station and pumping main together with associated land for access and compound will be required to serve HIMOR parcels Hi-C and Hi-D.

4.1.2 Redrow Homes

The masterplan foul water drainage strategy drawing 4072/SK110 can be located in Appendix I, highlighting the indicative location of the gravity foul water drainage, two possible pumping stations and pumping mains are based on the current Barton Willmore Concept Masterplan (Refer to Appendix B), together with outfall points to the United Utilities combined sewerage network.

Preliminary calculations have been undertaken based on the Sewers for Adoption allowance of 4,000 litres per dwelling per day and based on the current predicted level of development proposed of 332 properties. The estimated design flow for the Redrow Homes development site would be in the order of 15.4 l/s. However, this will be subject to review as the scheme is developed and therefore the proposed foul water system strategy has been developed with some flexibility to allow for the phased nature of the scheme and the potential for the masterplan to evolve over time.

The table below summarises the split of the properties based on the individual development site areas based on 34.6 dwellings/ha, and the estimated design foul flow rates.

Land Parcel	Area (ha)	Property No.	Flow Rate (I/s)
Redrow Homes	9.6	332	15.4

Based on the existing concept masterplan layout, development plot catchment plan (Refer to appendix E), existing site contour levels (Refer to Appendix C), the location and levels of the combined public sewerage network from the sewer record maps (Refer to Appendix D), it is currently assumed that two adoptable pumping stations and pumping mains together with



associated land for access and compound will be required to serve Redrow Homes parcels Red-A and Red-B.

4.1.3 VHW Land Partnership (Walshaw) Limited

The masterplan foul water drainage strategy drawing 4072/SK112 can be located in Appendix I, highlighting the indicative location of the gravity foul water drainage, a foul water pumping station and pumping main, together with sewer diversion works based on the current Barton Willmore Concept Masterplan (Refer to Appendix B), together with outfall points to the United Utilities combined sewerage network.

Preliminary calculations have been undertaken based on the Sewers for Adoption allowance of 4,000 litres per dwelling per day and based on the current predicted level of development proposed of 657 properties. The estimated design flow for the VHW Land Partnership (Walshaw) Limited development site would be in the order of 34.1 l/s, including and allowance of 3.7l/s for the one form entry primary school. However, this will be subject to review as the scheme is developed and therefore the proposed foul water system strategy has been developed with some flexibility to allow for the phased nature of the scheme and the potential for the masterplan to evolve over time.

The table below summarises the split of the properties based on the individual development site areas based on 34.6 dwellings/ha, and the estimated design foul flow rates.

Land Parcel	Area (ha)	Property No.	Flow Rate (I/s)
VHW-A	18.98	657	30.4
BC-SFES	-	-	3.7

Based on the existing concept masterplan layout, development plot catchment plan (Refer to appendix E), existing site contour levels (Refer to Appendix C), the location and levels of the combined public sewerage network from the sewer record maps (Refer to Appendix D), it is currently assumed that an adoptable pumping station and pumping main together with associated land for access and compound will be required to serve VHW Land Partnership (Walshaw) Limited land parcel VHW-I.

4.1.4 General Comments

Pre-development enquiries have been made for each site to United Utilities, and their responses which are valid for 12 months, can be located in Appendix D. United Utilities have confirmed that foul water from the sites are allowed to discharge unrestricted to the surrounding sewerage network and that connection points would be agreed at detailed application stage.



Notwithstanding the above, the foul water drainage strategy for the masterplan development site will be reviewed in more detail as part of the site wide engineering assessment. As such, during the development of the land parcels, alternative points of discharge to the public sewerage network at alternative locations may be required to suit the development proposals.

Given the phased approach of the development it will be necessary to agree the maintenance of the drainage system in the interim with Bury Council as United Utilities would only adopt sections of the drainage once it is receiving more than 50% development foul flows.

The above will be discussed and agreed as part of the S104 process with Bury Council and United Utilities respectively.



5.0 Surface Water Drainage

5.1 Introduction

The National Planning Policy Framework (NPPF) and accompanying Technical Guidance indicate that surface water run-off should be controlled as near to its source as possible through a sustainable drainage approach to surface water management.

Sustainable drainage (SUDS) techniques including soakaways, infiltration trenches, permeable pavements, grassed swales, ponds and wetlands should be employed wherever possible to help reduce flood risk by attenuating the rate and quantity of surface water run-off from a site. This approach can also offer other benefits in terms of promoting groundwater recharge, water quality improvement and amenity enhancements. Approved document Part H of the Building Regulations (2015) sets out a hierarchy for the disposal of surface water which encourages a SUDS approach.

CIRIA gives guidance and weighting to the various SUDS options that can be utilised within the further development of a masterplan at detailed design stage and to meet any possible planning requirements. Table 1.7 for the CIRIA document C697 'The SUDS Manual' is provided on the opposite page:



SUDS		Water quantity	Water quality	Environmental benefits	core
Technique Description		General Score	General Score	General Score	Total Score
Wetlands	As ponds, but the runoff flows slowly but continuously through aquatic vegetation that attenuates and filters the flow. Shallower than ponds.	75	100	100	92
Ponds	Depressions used for storing and treating water. They have a permanent pool and bankside emergent and aquatic vegetation.	63	100	100	88
Bioretention areas	Vegetated areas for collecting and treating water before discharge downstream, or to the ground via infiltration.	50	100	100	83
Green roofs	Vegetated roofs that reduce runoff volume and rate.	25	88	83	65
Pervious pavements	Allow inflow of rainwater into underlying construction/soil.	63	63	50	58
Swales Shallow	Vegetated channels that conduct and/or retain water (and can permit infiltration when un-lined). The vegetation filters particulates.	63	56	50	56
Infiltration basins	Depressions that store and dispose of water via infiltration.	50	50	50	50
Filter strips	Vegetated strips of gently sloping ground designed to drain water evenly from impermeable areas and filter out silt and other particulates.	38	50	50	46
Detention basin	Dry depressions designed to store water for a specified retention time.	25	44	50	40
Infiltration trenches	As filter drains, but allowing infiltration through trench base and sides.	63	50	0	38
Filter drain	Linear drains/trenches filled with a permeable material, often with a perforated pipe in the base of the trench.	50	50	0	33
Sand filters	Treatment devices using sand beds as filter media.	38	63	0	33
Soakaways	Sub-surface structures that store and dispose of water via infiltration.	25	38	0	21
Pipes, subsurface storage	Water quality can be targeted using		13	0	21
Silt removal devices	Manhole and/or proprietary devices to remove silt.	0	13	0	4
	Technique unsuitable for High Density Urba	n Environme	ents		
Based	on Table 1.7 of CIRIA C697 "The SUDS Manual"				



5.2 Pre-Development & Restricted Surface Water Run-Off

For the purposes of determining the existing rate of surface water run-off, the site is considered to be Greenfield.

The run-off rates from the permeable (greenfield) areas of the site have been calculated using MicroDrainage ICP SUDS which is based on the modified IOH124 using FSR Rainfall and adjusted for sites less than 50ha in accordance with the recommendations of the DEFRA R&D Technical Report SC030219 'Rainfall run-off management for development' (2013).

To determine more accurately the parameters for the SAAR (Statistical Average Annual Rainfall mm) and the Soil Type (SPR) across the masterplan development site, values from the UK SUDS Website <u>www.ukSUDS.com</u> were obtained from interrogating the maps available and taking the average reading for each landownership.

The UK SUDS greenfield run-off tool uses a digitised version of the Winter Rainfall Acceptance Potential (WRAP) map from the Wallingford Procedure which can be considered to provide more accurate SOIL and SPR values.

Individual parcel areas have been measured from the Barton Willmore Concept Masterplan (refer to Appendix B). Individual catchments as supplied by Barton Willmore are indicative only and will be subject to change as specific land parcels and development phasing are defined and further developed.

The total restricted discharge rate calculated within the three reports for the whole development site for the 30 year event is 367.6l/s and for the 100 year plus climate change (40%) event is 446.4l/s using the ICP SUDS method in Microdrainage (Innovyze).

5.2.1 HIMOR

For the current HIMOR catchment area, refer to the RoC Catchment Area drawing 4072/SK105 located in Appendix E.

The RoC Catchment Area drawing 4072/SK105 located in Appendix E relates solely to the masterplan drainage strategy for the current Barton Willmore Concept Masterplan and identifies the greenfield run-off for the proposed parcels and main proposed Link Road. Refer to Appendix F for the MicroDrainage Source Control Surface Water Greenfield Run-off Calculations.

The table below summarises the greenfield run-off rates generated by each proposed land parcel for a range of storm return periods and defines the limiting discharge flow rate requirements for the development for the 30year event and 100year plus 40% allowance for climate change. As a consequence, where flow rates are less than 5.0 l/s (i.e. 1.9*) then this has been taken as a minimum allowable discharge to mitigate against blockage from vegetation and other material.



The table below defines the proposed restricted discharge rates for the development parcels based on a 60% development area including an allowance of 10% for future development. Link Roads have been taken at 100%. The proposed use of SUDS will enable any variations within this strategy to be accommodated. These are subject to agreement with the LLFA.

Land	Gross Area	Net Area	Existing/Proposed Greenfield Runoff (I/s)					
Parcel	(ha)	(ha)	Q ₁	Q _{bar}	Q ₃₀	Q 100	(L/S) Q _{100+40%}	
Hi-A	3.65	2.19	16.2	18.7	31.6	38.8	38.8	
Hi-B	2.95	1.77	13.2	15.1	25.6	31.4	31.4	
Hi-C	0.62	0.38	2.8	3.2	5.4	6.6	6.6	
Hi-D	0.32	0.20	1.5	1.6	2.8*	3.4*	5.0	
Hi-R	0.44	0.44	3.3	3.7	6.4	7.8	7.8	

The total restricted discharge from the masterplan development site area is based on the above and would be 74.0l/s for the 30-year event and 89.6l/s for the 100 year plus climate change (40%) event.

Notwithstanding the above, further engineering assessment is required to agree the drainage strategy for the development parcels in conjunction with the proposed site levels. There may be a requirement for specific land parcels to discharge directly into one of the local water features or into the Link Road drainage network to mitigate against pumping of surface water. Should this occur then the discharge rate identified above would be amended accordingly.

5.2.2 Redrow Homes

For the current Redrow Homes catchment area, refer to the RoC Catchment Area drawing 4072/SK104 located in Appendix E.

The RoC Catchment Area drawing 4072/SK104 located in Appendix E relates solely to the masterplan drainage strategy for the current Barton Willmore Concept Masterplan and identifies the greenfield run-off for the proposed parcels and main proposed Link Road. Refer to Appendix F for the MicroDrainage Source Control Surface Water Greenfield Run-off Calculations.



The table below summarises the greenfield run-off rates generated by each proposed land parcel for a range of storm return periods and defines the limiting discharge flow rate requirements for the development for the 30year event and 100year plus 40% allowance for climate change. As a consequence, where flow rates are less than 5.0 l/s (i.e. 1.9*) then this has been taken as a minimum allowable discharge to mitigate against blockage from vegetation and other material.

The table below defines the proposed restricted discharge rates for the development parcels based on a 60% development area including an allowance of 10% for future development. Link Roads have been taken at 100%. The proposed use of SUDS will enable any variations within this strategy to be accommodated. These are subject to agreement with the LLFA.

Land	Gross Area Net Area		Existing/Proposed Greenfield Runoff (I/s)				Maximum Discharge
Parcel	Parcel (ha)	(ha)	Q ₁	Q _{bar}	Q ₃₀	Q 100	(L/S) Q _{100+40%}
Red-A	7.48	4.49	32.3	37.1	63.0	77.2	77.2
Red-B	2.00	1.20	8.6	9.9	16.8	21.8	21.8
Red-C	0.12	0.08	0.5	0.6	1.0*	1.3*	5.0
Red-R	0.11	0.11	0.8	0.9	1.5*	1.9*	5.0

The total restricted discharge from the masterplan development site area is based on the above and would be 89.8/s for the 30-year event and 109.0/s for the 100 year plus climate change (40%) event.

Notwithstanding the above, further engineering assessment is required to agree the drainage strategy for the development parcels in conjunction with the proposed site levels. There may be a requirement for specific land parcels to discharge directly into one of the local water features or into the Link Road drainage network to mitigate against pumping of surface water. Should this occur then the discharge rate identified above would be amended accordingly.

5.2.3 VHW Land Partnership (Walshaw) Limited

For the current VHW Land Partnership (Walshaw) Ltd catchment area, refer to the RoC Catchment Area drawing 4072/SK106 located in Appendix E.

The RoC Catchment Area drawing 4072/SK106 located in Appendix E relates solely to the masterplan drainage strategy for the current Barton Willmore Concept Masterplan and identifies



the greenfield run-off for the proposed parcels and main proposed Link Road. Refer to Appendix F for the MicroDrainage Source Control Surface Water Greenfield Run-off Calculations.

The table below summarises the greenfield run-off rates generated by each proposed land parcel for a range of storm return periods and defines the limiting discharge flow rate requirements for the development for the 30year event and 100year plus 40% allowance for climate change. As a consequence, where flow rates are less than 5.0 l/s (i.e. 1.9*) then this has been taken as a minimum allowable discharge to mitigate against blockage from vegetation and other material.

The table below defines the proposed restricted discharge rates for the development parcels based on a 60% development area including an allowance of 10% for future development. Link Roads have been taken at 100%. The proposed use of SUDS will enable any variations within this strategy to be accommodated. These are subject to agreement with the LLFA.

Land	Gross Area	Net Area	Existing/Proposed Greenfield Runoff (I/s)				Maximum Discharge
Parcel (ha)	(ha)	Q ₁	Q _{bar}	Q ₃₀	Q 100	(L/S) Q _{100+40%}	
VHW-A	1.12	0.67	4.9	5.6	9.5	11.8	11.8
VHW-B	3.96	2.38	17.3	19.9	33.8	41.5	41.5
VHW-C	2.20	1.32	9.66	11.1	18.8	23.0	23.0
VHW-D	0.72	0.43	3.2	3.6	6.1	7.6	7.6
VHW-E	1.80	1.08	7.9	9.1	15.4	18.8	18.8
VHW-F	0.24	0.15	1.1	1.2	2.1*	2.5*	5.0
VHW-G	3.62	2.17	15.8	18.2	30.9	37.9	37.9
VHW-H	2.39	1.62	11.8	13.6	23.0	28.2	28.2
VHW-I	2.63	1.58	11.5	13.3	22.4	27.5	27.5
VHW-R1	0.47	0.47	3.4	3.9	6.7	8.2	8.2
VHW-R2	0.26	0.26	1.9	2.2	3.7*	4.5*	5



VHW-R3	0.63	0.63	4.6	5.3	9.0	11.0	11.0
BC-SFES	2.13	1.28	9.4	10.8	18.2	22.3	22.3

The total restricted discharge from the masterplan development site area is based on the above and would be 203.8l/s for the 30-year event and 247.8/s for the 100 year plus climate change (40%) event.

Notwithstanding the above, further engineering assessment is required to agree the drainage strategy for the development parcels in conjunction with the proposed site levels. There may be a requirement for specific land parcels to discharge directly into one of the local water features or into the Link Road drainage network to mitigate against pumping of surface water. Should this occur then the discharge rate identified above would be amended accordingly.

5.3 Methods of Surface Water Management

The site is currently undeveloped greenfield land and can be considered 100% permeable however, this will change post development through the introduction of proposed buildings, access roads and area of hardstanding.

There are three methods that have been reviewed for the management and discharge of surface water which are detailed below; these may be applied individually or collectively to form a complete strategy. They should be applied in the order of priority as listed.

- Discharge via infiltration
- Discharge to watercourse
- Discharge to public sewerage system

Discharge via Infiltration

Any impermeable areas that can drain to a soakaway or an alternative method of infiltration would significantly improve the sustainability of any surface water systems.

It is understood that to date, no soakaway testing has been carried out on the masterplan development site.

Reference to the online British Geological Survey (BGS) map for the site indicates the following ground model:

Bedrock Geology: Pennine Lower Coal Measures – Mudstone, Siltstone, and Sandstone. Interbedded with Cannel Rock (South Lancashire) – Sandstone. Sedimentary bedrock formed approximately 319 million years ago in the Carboniferous Period



Superficial Deposits: Predominantly Devensian – Diamicton Till, with a small area of Glaciofluvial Deposits, Devensian – Sand and Gravel in the Western area of the site that encompasses the Eastern most pond / lake. Superficial Deposits formed up to 2 million years ago in the Quaternary Period

The Bury Local Flood Risk Management Strategy (2018) suggests that the geology 'consisting predominately of sand and gravels which have high permeability. However, there are areas of clay which have low permeability'.

Soilscape England describes the soils as 'slowly permeable, seasonally wet acid loamy and clayey with impeded drainage.'

Based on the underlying geology it is considered that infiltration could potentially be a suitable method of surface water disposal. Subject to intrusive ground investigation confirming the underlying geology is suitable in principle, soakaway testing should be carried out in accordance with BRE365 to determine whether any infiltration solution can potentially be applied as a feasible method of surface water management.

If favourable infiltration rates are returned from the site investigation, surface water arising from the proposed development will be discharged via one or more infiltration solutions such as soakaways or permeable pavements, the design standard will be no surface flooding for storm events up to and including the 30 year return period in accordance with the SUDS Manual (2015).

The resultant storage volume depends on the infiltration rate and contributing impermeable area for each soakaway/area of permeable paving; this will be determined at detailed design stage when the proposed masterplan has been developed further.

However, it should be noted from the Bury Local Flood Risk Management Strategy referenced to in the Flood Risk Assessments, identify that the land in general is shown to be at varying risk from >=25%<50% and >75% risk of groundwater flooding. As such, it may not be practical to consider infiltration as a practical solution as this could increase the risk of ground waterflooding across the site.

In addition, the groundwater level is unknown and the requirement for soakaways, infiltration structures are to be set at least 1m above the highest recorded winter water level, together with the unknown infiltration rate and risk of groundwater flooding, which makes it highly unlikely that it will be practical to rely on infiltration as a suitable means of surface water control from development run-off.

5.3.1 HIMOR

Discharge to Watercourse

As indicated earlier in this report, there are three ordinary watercourse features in close proximity to the site. These are an un-named watercourse to the north of Scobell Street outside the



masterplan boundary, Walshaw Brook within the northern area of the masterplan site, i.e. to the northern boundary of VHW Land Partnership (Walshaw) Limited and southern boundaries for the HIMOR and Redrow Homes sites and Elton Brook just outside the masterplan southern boundary i.e. south of Walshaw Lane.

Based on the topography of the site and subject to confirmation of proposed site levels, it is anticipated that the west and central parts of the HIMOR site, land Parcels Hi-B, Hi-C and Hi-R will drain to Walshaw Brook, with the topography generally falling south-east from 133m to 123m AOD and south-west 126m to 121m AOD respectively.

All surface water discharge to ordinary watercourses and/or tributaries will need to be restricted to greenfield run-off rates, subject to agreement with the Lead Local flood Authority.

Discharge to Public Sewer

If it is ultimately determined that discharge via infiltration or to either of the ordinary watercourses or tributaries is not feasible, then it will be necessary to look at discharging those parts of the sites into the public surface water sewer network.

Any discharge to a public sewer system would be subject to confirmation of invert levels, capacity and condition. United Utilities and the LLFA would need to be consulted at the appropriate time to discuss and agree discharge rates and suitable points of connection to the system.

The east part of the HIMOR site, Parcel Hi-A, following the current natural levels generally falls in a south east direction from 121m to 118m AOD. However, it may be difficult to obtain a gravity connection to the south and therefore an alternative connection to the existing culverted watercourse on the northern side of Scobell Street could be considered as an alternative. This measures 1.5m diameter and flows in a westerly direction along the frontage of the existing residential properties. From review of the sewer records a connection could be made downstream of MH 2104 which has an invert level of 114.92m AOD.

Discharge to the United Utilities sewerage network would be restricted to greenfield run-off rates or less, depending on the available capacity and may only be agreed once all other options for disposal have been exhausted. Appropriate evidence would need to be provided to demonstrate that infiltration and a connection to any local watercourse are not feasible methods of disposal.

5.3.2 Redrow Homes

Discharge to Watercourse

As indicated earlier in this report, there are three ordinary watercourse features in close proximity to the site. These are an un-named watercourse to the north of Scobell Street outside the masterplan boundary, Walshaw Brook within the northern area of the masterplan site, i.e. to the northern boundary of VHW Land Partnership (Walshaw) Limited and southern boundaries for the HIMOR and Redrow Homes sites and Elton Brook just outside the masterplan southern boundary i.e. south of Walshaw Lane.



Based on the topography of the site and subject to confirmation of proposed site levels it is anticipated that the Redrow Homes site will drain by gravity to Walshaw Brook. The topography of the site generally slopes in a southern direction, with levels ranging from circa 120m AOD in the north to circa 115m AOD in the south, with land parcels Red-A and Red-C draining southwards and parcel Red-B draining south westerly.

All surface water discharge to ordinary watercourses and/or tributaries will need to be restricted to greenfield run-off rates, subject to agreement with the Lead Local flood Authority.

Discharge to Public Sewer

If it is ultimately determined that discharge via infiltration or to either of the ordinary watercourses or tributaries is not feasible, then it will be necessary to look at discharging those parts of the sites into the public surface water sewer network.

The nearest available surface water asset is the culverted watercourse to the north of Scobell Street. Based on the topography of the site it is anticipated that a pumped connection would be required.

Any discharge to a public sewer system would be subject to confirmation of invert levels, capacity and condition. United Utilities and the LLFA would need to be consulted at the appropriate time to discuss and agree discharge rates and suitable points of connection to the system.

Discharge to the United Utilities sewerage network would be restricted to greenfield run-off rates or less, depending on the available capacity and may only be agreed once all other options for disposal have been exhausted. Appropriate evidence would need to be provided to demonstrate that infiltration and a connection to any local watercourse are not feasible methods of disposal.

5.3.3 VHW Land Partnership (Walshaw) Limited

Discharge to Watercourse

As indicated earlier in this report, there are three ordinary watercourse features in close proximity to the site. These are an un-named watercourse to the north of Scobell Street outside the masterplan boundary, Walshaw Brook within the northern area of the masterplan site, i.e. to the northern boundary of VHW Land Partnership (Walshaw) Limited and southern boundaries for the HIMOR and Redrow Homes sites and Elton Brook just outside the masterplan southern boundary i.e. south of Walshaw Lane.

Based on the topography of the site and subject to confirmation of proposed site levels it is anticipated that the northern part of the VHW Land Partnership (Walshaw Limited) site from Parcels VHW-A to VHW-E and VHW-R1 & R2 will drain to Walshaw Brook, with the topography generally falling north-east from 140m to 117m AOD. The southern section of the site from Parcels VHW-F to VHW-I and VHW-R3, including Bury Council Parcel BC-A will drain to Elton Brook via the onsite tributaries, with the topography generally falling north-east from 140m to 110m AOD.



All surface water discharge to ordinary watercourses and/or tributaries will need to be restricted to greenfield run-off rates, subject to agreement with the Lead Local flood Authority.

Discharge to Public Sewer

If it is expected that a connection to the existing watercourse network will be feasible and therefore this method of disposal has not been considered further.

5.4 Surface Water Drainage Strategy

As described above, if discharge via infiltration is not feasible then it will be necessary to restrict discharge from the development parcels and Link Road sections to the corresponding greenfield run-off rate.

Therefore, the method for surface water disposal for the proposed masterplan development sites, is where practical to discharge directly to watercourse in accordance with the hierarchy for surface water disposal as set out in Part H of the Building Regulations and in accordance with the NPPF. Based on the Barton Willmore Concept Masterplan, the development site has been sub-divided into land parcels and Link Road sections, following the requirement for each landownership to be brought forward independently with the drainage run-off calculated on a parcel by parcel basis.

The outline surface water drainage strategy is based on the assumption that each development parcel of land or Link Road section will be restricted to the equivalent greenfield run-off, with attenuation provided to protect the development sites from flooding for up to the critical 100 year event including 40% for climate change, in accordance with the requirements of the' Non-Statutory Sustainable Technical Standards' for peak flow control ensuring that the overall proposed development would not exceed the existing scenario and a betterment would be provided for larger return periods.

As this is an outline drainage strategy no allowance has been made for attenuation within the land parcel drainage systems or the betterment provided by the implementation of SUDS systems.

As such the attenuation volumes that have been calculated for the development land parcels have been based on the restricted greenfield run-off rates and assumed impermeable areas utilising high level quick storage estimate with the upper limits specified. This approach will give a conservative approach.

The general principle of the surface water drainage strategy is to provide new drainage systems serving the proposed impermeable development areas with discharge to the various existing ordinary watercourse features within the development boundary wherever practically possible, or alternatively to off-site surface water sewers or, combined public sewer as a last option. Given the varying level nature of the development site and independent sites, subject to topographical surveys, surveys of the existing watercourses and ditches and the setting of external levels at



detail design stage, one or more pumping stations may be required, these could be offered to United Utilities or the LLFA for adoption.

The development site has been split up along the lines of landownership to allow for independent development, with each landownership area being sub-divided into several notional catchments to suit the Barton Wilmore Concept Masterplan and existing drainage features. The discharge rate from each of these catchments have been restricted to the equivalent 30year and 100year greenfield run-off rates, or a minimum of 5l/s as a practical rate for small catchments. Attenuation volumes were estimated using the Microdrainage Quick Storage Estimate (QSE) tool with a combination of detention basins, swales, geo-cellular tanks and/or oversized pipework suggested to meet these requirements.

It is assumed that as each phased parcel of land is brought forward for development on site, SUDS will be considered and incorporated if practical, such as rain gardens, water butts, permeable paving for drives etc prior to connecting into the overall infrastructure.

For the proposed new Link Roads, it has been suggested where practical and subject to the agreement of the adopting authority(s) that they could be drained by parallel swales, or with Rain Gardens/Bio-remediation where tree-lined, or oversized pipework laid beneath the highway which again would discharge if practical to one of the existing watercourses within the site or alternatively to off-site surface water sewers or, combined public sewer as a last option.

The total restricted discharge rate calculated within the three reports for the whole development site for the 30 year event is 367.6l/s and for the 100 year plus climate change (40%) event is 446.4l/s using the ICP SUDS method in Microdrainage (Innovyze).

The total attenuation volume calculated within the three reports for the 30 year was 9,146m³ and for the 100 year plus climate change (40%) event was 19,063m³ using the upper limit of the quick Storage Estimate from Source Control in Microdrainage (Innovyze). The upper limit was used to make allowance for the 100year 6Hr volume long term storage volume to be calculated and confirmed at detail design stage.

An existing ordinary watercourse defined as Walshaw Brook is present within the development boundary generally flowing in a southeast direction, including a number of reservoirs/ponds and open and culverted tributaries. Elton Brook also an existing ordinary watercourse which generally flows in an easterly direction with a downstream series of reservoirs and is located outside the site development boundary to the south of Walshaw Lane. However, there is a main open and culverted tributary of Elton Brook within the development site which crosses a number of the development parcels. As a result of the development proposals a number of these are affected and would need to be either culverted, diverted or abandoned.

Existing watercourse and ditches receive development run-off from outside the site development boundary and ultimately need to be maintained. It is proposed that these are further investigated, surveyed and hydraulically modelled to inform their current operational capacity and condition.



Where necessary these will need to be cleaned out and regraded to ensure that base flows and current biodiversity are maintained.

The outline total attenuation volume required for the Masterplan Development Site Area based on the summation of the tables below would be 9,665m³ for the 30 year event and 19,063m³ for the 100 year plus climate change (40%) event, based on the proposed restricted greenfield run-off rates mentioned in Section 5.2.

5.4.1 HIMOR

The masterplan surface water drainage strategy drawing 4072/SK108 highlighting the indicative location of the detention basins, swales, rain garden/bio-remediation and possible on-line attenuation can be located in Appendix H.

The table below summarises the attenuation volumes for each development parcel or Link Road section, against the proposed restricted greenfield run-off rates. The Microdrainage calculations for the proposed attenuation can be located in Appendix G.

Land	Imp	Proposed Greenfield Runoff Rates & Attenuation Volumes					
Parcel	Area (ha)	Max. Discharge Q ₃₀ (I/s)	Max. Discharge Q _{100+40%} (I/s)	Volume Q ₃₀ (m ³)	Volume Q _{100+40%} (m ³)		
Hi-A	2.19	31.6	38.8	827	1677		
Hi-B	1.77	25.6	31.4	667	1355		
Hi-C	0.38	5.4	6.6	144	293		
Hi-D	0.20	-	5.0*	-	136		
Hi-R	0.44	6.4	7.8	166	337		

The outline total attenuation volume required for the Masterplan Development Site Area based on the above table would be 1,940m³ for the 30-year event and 3,798m³ for the 100 year plus climate change (40%) event.

As noted earlier in this document, further engineering assessment is required to agree the drainage strategy for these land parcels at detail design stage as the land parcels are brought forward with developed layouts and external levels and reserved matter are addressed.



5.4.2 Redrow Homes

The masterplan surface water drainage strategy drawing 4072/SK107 highlighting the indicative location of the detention basins, swales, rain garden/bio-remediation and possible on-line attenuation can be located in Appendix H.

The table below summarises the attenuation volumes for each development parcel or Link Road section, against the proposed restricted greenfield run-off rates. The Microdrainage calculations for the proposed attenuation can be located in Appendix G.

	Imp	Proposed Greenfield Runoff Rates & Attenuation Volumes						
Land Area Parcel (ha)		Max. Discharge Q ₃₀ (I/s)	Max. Discharge Q _{100+40%} (I/s)	Volume Q ₃₀ (m ³)	Volume Q _{100+40%} (m ³)			
Red-A	4.49	63.0	77.2	1713	3475			
Red-B	1.20	16.8	21.8	458	911			
Red-C	0.08	-	5.0*	-	38			
Red-R	0.11	-	5.0*	-	60			

The outline total attenuation volume required for the Masterplan Development Site Area based on the above table would be 2,269m³ for the 30-year event and 4,484m³ for the 100 year plus climate change (40%) event.

As noted earlier in this document, further engineering assessment is required to agree the drainage strategy for these land parcels at detail design stage as the land parcels are brought forward with developed layouts and external levels and reserved matter are addressed.

5.4.3 VHW Land Partnership (Walshaw) Limited

The masterplan surface water drainage strategy drawing 4072/SK109 highlighting the indicative location of the detention basins, swales, rain garden/bio-remediation and possible on-line attenuation can be located in Appendix H.

The table below summarises the attenuation volumes for each development parcel or Link Road section, against the proposed restricted greenfield run-off rates. The Microdrainage calculations for the proposed attenuation can be located in Appendix G.



	Imp	Proposed Greenfield Runoff Rates & Attenuation Volumes						
Land Parcel	Area (ha)	Max. Discharge Q ₃₀ (I/s)	Max. Discharge Q _{100+40%} (I/s)	Volume Q ₃₀ (m ³)	Volume Q _{100+40%} (m ³)			
VHW-A	0.67	9.5	11.8	254	514			
VHW-B	2.38	33.8	41.5	903	1833			
VHW-C	1.32	18.8	23.0	501	1017			
VHW-D	0.43	6.1	7.6	163	330			
VHW-E	1.08	15.4	18.8	409	832			
VHW-F	0.15	-	5.0*	-	92			
VHW-G	2.17	30.9	37.9	823	1670			
VHW-H	1.62	23.0	28.2	615	1249			
VHW-I	1.58	22.4	27.5	600	1218			
VHW-R1	0.47	6.7	8.2	178	362			
VHW-R2	0.26	-	5.0*	-	193			
WWH-R3	0.63	9.0	11.0	239	485			
BC-SFES	1.28	18.2	22.3	486	986			

The outline total attenuation volume required for the Masterplan Development Site Area based on the above table would be 5,456m³ for the 30-year event and 10,781m³ for the 100 year plus climate change (40%) event.

As noted earlier in this document, further engineering assessment is required to agree the drainage strategy for these land parcels at detail design stage as the land parcels are brought forward with developed layouts and external levels and reserved matter are addressed.



The existing ordinary watercourse network will be maintained and enhanced where possible through the masterplan development site with sections of watercourse/ditch abandoned, diverted and/or culverted as required where they conflict with the proposed development parcels and/or Link Road sections.

Work to assess the wider watercourse network and its implications on the development parcels, including hydraulic modelling and drainage surveys which will be undertaken as part of the engineering assessment noted above. This will be discussed in further detail with Bury Council as Lead Local Flood Authority.

Subject to further discussions, it is proposed to offer the surface water drainage system for adoption to United Utilities under S104 of the Water Industry Act. This will only be achievable where the drainage system receives development run-off from adjacent land parcels. Where the system is only draining the Link Road, the system will be offered to Bury Council as the highway authority who would also be responsible for the proposed rain garden/bio-remediation, together with the more traditional road gullies and connections.

Given the phased approach of the development, it will be necessary to have discussions with Bury Council and United Utilities as the surface water drainage strategy is developed to discuss and agree what will be acceptable and who will take ownership and maintenance of the various elements of the drainage systems and attenuation facilities.

In addition, due to the phased nature, discussions and agreement will also be required as to who will be responsible for the drainage systems and attenuation facilities in the interim with Bury Council, as United Utilities are likely to only adopt sections of the drainage once it is receiving a minimum 50% of development run-off.

The above will be discussed and agreed as part of the S38/S104 process with Bury Council and United Utilities respectively.

6.0 Summary

This summary relates to the outline masterplan drainage strategy associated with the proposed residential developments and a one form entry primary school to be brought forward independently by the separate four landownerships comprising HIMOR, Redrow Homes, VHW Land Partnership (Walshaw) Limited and Bury Council.

The outline masterplan drainage strategy has been developed to provide an overview for the whole 64ha development site and bring together the general principles developed in the RoC Flood Risk Assessments and Outline Drainage Strategies for outline masterplan planning stage.



Separate foul and surface water drainage networks are proposed within the land parcels and beneath the Link Road which are intended to serve both the primary infrastructure and the adjacent development parcels. Points of connection and flow rates have yet to be agreed and are subject to formal consent and technical approval with the LLFA and United Utilities.

Primary surface water attenuation is proposed in the form of a combination of detention basins, rain garden/bio-remediation, swales, geo-cellular tanks and/or oversized pipework to be developed for each land parcel and Link Road at detail design stage. Supported by the incorporation of suitable SUDS techniques as practical within the development land parcels.

Gravity foul water drainage is proposed to serve the individual land parcel developments and the site development. To meet the requirement of site levels, pumping stations and pumping mains may be required in low lying areas.

It is proposed that both the foul and surface water drainage systems will be offered for adoption to United Utilities under Section 104 of the Water Industry Act, with any surface water drainage which does not receive development flows being adopted by Bury Council as highway drainage through the S38 adoption process.

As this masterplan drainage strategy is based on a concept masterplan layout to support an application for the allocation of the site through the GMSF, additional site Investigation, drainage surveys, hydraulic modelling and further engineering assessments will need to be undertaken to further develop the MDS as the site masterplan id developed. This will further inform the land parcel layout, levels, drainage and the wider development. This will impact on the current Barton Willmore Concept Masterplan for the Walshaw Garden Neighbourhood Development Framework and outline masterplan concept drainage strategy scheme and therefore this report is subject to change.



APPENDIX A – BARTON WILLMORE LOCATION AND LAND OWNERSHIP PLAN





APPENDIX B – BARTON WILLMORE CONCEPT MASTERPLAN





APPENDIX C – EXISTING TOPOGRAPHY PLANS


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<u>NOTES</u>

- ALL DIMENSIONS ARE IN METRES UNLESS NOTED OTHERWISE.
- THIS DRAWING SHOULD BE READ IN CONJUNCTION WITH THE ROC FLOOD RISK ASSESSMENT REPORT AND MASTERPLAN DRAINAGE STRATEGY REPORT.
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KEY

- 5m CONTOUR LINE 1m CONTOUR LINE

OVERLAND FLOW ROUTE OFFSITE OVERLAND FLOW ROUTE

RED LINE BOUNDARY

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Redrow Homes

PROJECT TITLE

CLIENT

WALSHAW GARDEN NEIGHBOURHOOD BURY

DRAWING TITLE **EXISTING TOPOGRAPHY & OVERLAND FLOW ROUTES** SHEET 1 OF 3

PROJECT No. DRAWING No. REV. 4072 SK101 01 C RoCp Ltd.







COMPLEX CHALLENGES ... MADE SIMPLE

APPENDIX D – SEWER RECORDS

Reece McGuinness

From: Sent:	Wastewater Developer Services <wastewaterdeveloperservices@uuplc.co.uk> 06 January 2020 13:14</wastewaterdeveloperservices@uuplc.co.uk>
То:	Reece McGuinness; Wastewater Developer Services
Subject:	RE: File Transfer: 4072 - Land at Walshaw Bury Pre-Development Enquiry (HIMOR) - 4200029694

Dear Customer

We have carried out an assessment of your application which is based on the information provided; this pre development advice will be valid for 12 months

Foul will be allowed to drain to the public combined sewer network at an unrestricted rate. The connection(s) to the public sewer can be at a point(s) convenient to yourself

Surface water from this site should drain to either soak away/infiltration system or directly to watercourse. Discharge rates and consents must be discussed and agreed with all interested parties.

If you require any further guidance please follow http://www.unitedutilities.com/builders-developers.aspx

If you need a make further enquiry relating to this matter please send your enquiry to <u>WastewaterDeveloperServices@uuplc.co.uk</u> Please quote your DEXXXX/42XXXXX/GEXXXX reference number

Please Note:- enquiries sent to any other United Utilities e-mail address will be deleted.

Connection Application

Although we may discuss and agree discharge points & rates in principle, please be aware that you will have to apply for a formal sewer connection. This is so that we can assess the method of construction, Health & Safety requirements and to ultimatley inspect the connection when it is made. Details of the application process and the form itself can be obtained from our website by following the link below

http://www.unitedutilities.com/connecting-public-sewer.aspx

Please be aware that on site drainage must be designed in accordance with Building Regulations, National Planning Policy, Planning Conditions and local flood authority guidelines, we would recommend that you laise and make suitable agreements with the relevant statutory bodies.



Neil O'Brien Wastewater Pre-Development Engineer Great Manchester Area Developer Services & Metering Customer Services T: 01925 679410 unitedutilities.com

From: Reece McGuinness [mailto:reece.mcguinness@rocconsulting.com]
Sent: 24 December 2019 12:33
To: Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>
Subject: File Transfer: 4072 - Land at Walshaw Bury Pre-Development Enquiry (HIMOR) - Land at Walshaw, Bury

IMPORTANT: Click a link below to access files associated with this transmittal that came in through the RoC Consulting Info Exchange web site. <u>Download all associated files</u>

Additional links: **Reply to All** Project Land at Walshaw, Bury Name: Project 4072 Number: From: **Reece McGuinness** To: WastewaterDeveloperServices@uuplc.co.uk CC: David Eato; Paul White (RoC Consulting) 4072 - Land at Walshaw Bury Pre-Development Enquiry (HIMOR) Subject: Sent via: Info Exchange **Expiration** 1/23/2020 Date: Remarks: Dear Sir/Madam, Please find attached pre-development enquiry for the above site. Also attached is a plan indicating layout and an ownership plan. this pre-development enquiry relates to the HIMOR land. **Kind regards Reece McGuinness Strategic Land Graduate Engineer** T 0161 214 5390 www.rocconsulting.com



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			PM	КВ
30860-MR-M-021-Block	JPEG Image	12/24/2019	12:05	6,446
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<u>01.jpg</u>				
<u>30860-MR-M-04-Land</u>	JPEG Image	12/24/2019	12:06	12,405
Ownership Plan-			PM	КВ
A1_PT_1.2500-01.jpg				
Himor Pre Development	PDF File	12/24/2019	12:22	1,457
Enquiry.pdf			PM	КВ

To share and learn more about Newforma Info Exchange visit: Newforma Community Site

Kind regards Reece McGuinness Strategic Land Graduate Engineer T 0161 214 5390 www.rocconsultina.com



EMGateway3.uuplc.co.uk made the following annotations

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* Step Step<	Abardoned Foul Surface Water Combined Abardoned Foul Surface Water Combined Privata Saw Section 104 String Main Bituge Main Overflow Section 104 All point assets follow the standard colour convention: Water Court Manhole String Main Bituge Main Overflow Manhole Stille Entry Manhole Change of Survey Stille Entry Manhole Bituge Main Outriel Boding Eye Stille Entry Manhole Bituge Main Outriel Bituge Main Stille Entry Manhole Bituge Main Outriel Bituge Main Stille Entry Manhole Bituge Main Outriel Bituge Main Stille Entry Manhole Bituge Main Stille Entry Manhole Bituge Main Outriel Bituge Main Stille Entry Manhole Bituge Main Outriel Bituge Main Outriel Bituge Main Outriel Bituge Main Outriel Bituge Main Outriele
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irt	Size x	Size y	Shape	Mati	Length	Grad
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79	450			co	37.60959	1 in 85
41	939			co	26.56193 37,04357	1 in 47
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26	450			00	44,11641 50,53435	1 in 17
86	450 939 939			00	43.95328	1 in 17 1 in 126 1 in 97 1 in 103 1 in 36
35	450 150 100 150 225 225 150 375			co	20.64111	1 in 103
23	100			VC	34,40716	1 in 36
49 36	150 225			VC	17.09214 75.60423	1 in 31
	225			VC	22.80351 9.876245	
08 68 63	375			co	35.4683	1 in 68 1 in 130
63	300 225 100 100 225 225 225			VC	22.09072	
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77 23	450 225			CO VC	19.64998 32.14328	1 in 6 1 in 140
79 31	450			CO	31.6363	1 in 66 1 in 28
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	100			VC	10,18797 9,323336	
	100			VC	9.323336	
	100			VC	5.787055	
	100			VC	11,16586	
26 38	375			co	35,73514 35,44009	1 in 143 1 in 111
1	225			VC	11,28121 36,79674	
	100			VC	8.575326	
81 31 01	939			co	12.93917	1 in 26 1 in 44 1 in 19
01	225			VC	42.3201	1 in 19
	0			VC	6.908939	
94 .11	300			co	51.61395 46.87217	1 in 29 1 in 33
	300			CO	5.141829 49,1935	1 in 33
91 64 36 .9 33 74 02 94 01 05 .94 05	225 300			CO	50.08992 32.44996	1 in 33 1 in 30 1 in 54 1 in 89 1 in 951 1 in 247 1 in 85 1 in 259 1 in 33 1 in 33 1 in 19
.9	225			VC	26.62705 38.0526	1 in 89
74	375			CO	37.05402	1 in 247
94	300			co	75,54935	1 in 85
05	300			co	43.78757	1 in 33
.06	225			VC	67.41157 25.12753	1 in 33 1 in 19
	225			VC	15.65248 22.02271	
	225			VC	17.11724	
	225			co	24.73863	
	100			VC	11.51569	
23 02 .33 .33 .33	225			VC	56.88585	1 in 39 1 in 58 1 in 210 1 in 136
.33	225			VC	56.71861 38.20995	1 in 210 1 in 136
.33	225			UN	31.257 5.514241	
.13	150 225			VC	10,83996 6,082763	
13 14	225			VC	64.60784	
29	175			CI	49,40789	
	100			VC	12.86955	
62 .05	225			VC	41.23771	1 in 107
.05	100			VC	33.12099	1 in 107
	100			VC	10.39799 3,693306	
	225 225 225 225 225 225 225 225 225 225			VC	22.79353 6.981816	
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.72	225			VC	30.23243 41.64567 15.26434 11.70322	1 in 432
	225			VC	15,26434	
	100			VC	11,70322	

LEGEND Foul Surface Water Combin Public Sewer Private Sewer Section 104 Rising Main Sludge Main Overflow Water Course 1 in 17 1 in 126 1 in 97 1 in 103 1 in 36 1 in 68 1 in 95 1 in 95 1 in 162 1 in 162 brown - foul Manhole Side Entr 1 in 151 1 in 148 1 in 43 1 in 27 1 in 27 1 in 161 1 in 161 1 in 66 1 in 28 1 in 30 1 in 20 Head of System Extent of Survey Rodding Eye Inlet Outlall Screen C inspection Char all Ilact all Discharge Point all Discharge Point all Penstock all Nenstock all Nan Reburn Val all Soalaway all Non Reburn Val all Soalaway all Flow Meter all Dinterceptor all Dinterceptor all Din pop Shaft all Ding Shaft all Ding Shaft Lamp Hole Lamp Hole T Junction / Saddle Catchpit Valve Chamber Vent Column Vortex Chamber Vortex Chamber Penatock Chamber Network Storage Tank Sewer Overflow Ww Treatment Works Ww Pumping Station Septic Tank Change of Cha Crifice Plate FC Foul SW Surface Wat CO Combined OV Overflow 1 in 39 1 in 58 1 in 210 1 in 136 SEWER SHAP CI Circular TR Tra EG Egg AR Arch OV Oval BA Barrel FT Flat Top HO HorseSho RE Rectangular UN Unspecifie SQ Square SEWER MATERIAL Asbestos Cement BR Brick PE Polyethylene RP Reinforced Plan CO Concrete CSB Concrete Seg CSU Concrete Segment Unbolt CC Concrete Box Culverted PSC Plastic / Steel Compos GRC Glass Reinforecd Plaste Ductile Iron PVC Polyvinyl Chloride CI Cast iron Spun Iron Steel VC Vitrified Clar PP Polypropylen Pitch Fibre MAC Masonry Courses MAR Mesonry Random U Unspecified Address or Site Reference: 467 WALSHAW ROAD BURY, BL8 3AA OS sheet SD7710NE Number: Scale: 1:1250 Date: 19/04/2018 Nodes: 319 Sheet: 3 of 8 Printed by: Property Searches United SEWER RECORDS "Ping life flow smoothly























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EG Eng AR Arch O' O' O' Bail Barrel FF Field Top 'U' HouseShow RE Rectanguer UN Unspective SO Square SPERMATEMAL AC Addensor Cenner BR Bink PE Polyshytem RP Reinforcer Plants Lathin CO Concrete Boar Cathene COS Concrete Boar Cathene Proc Polympi Choine COS Concrete Boar Cathene Brow Polympi Choine Concrete Boar Cathene SI Souri To Cost from SI Souri To SI Souri To Cost from SI Souri To SI Souri To Cost from SI Souri To Cost from SI Souri To Cost from SI Souri To SI Souri To SI Souri To Cost from SI Souri To SI Souri To Cost from SI Souri To SI Souri To SI Souri To Cost from SI Souri To SI Souri To SI Souri To SI Souri To Cost from SI Souri To SI Souri To Cost from SI Souri To SI Souri To Cost from SI Souri To SI Souri To							SEWER SHAPE
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BR Brick PF Paystrytene RP Reinforced Plastic Mathie CC Concrete CSB Concrete Segment Unbolted CC Concrete Segment Segm							SEWER MATERIAL
PE Polyethylene RP Reinforced Plasts: Mathie CS Concrete Segment Belled CSU Concrete Segment Urbohed CC Concrete Segment							
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GRC Glass Reinforeid Plastic D Ducilie from PPC Poyviny/ Orkinde C Gait from SI Spun from ST Steel VC Verified Cuy PP Potypopylene PF Pitch Fibre MAC Masony, Random U Unspecified Address or Site Reference: Land at Walshaw Bury, Scale: 1:1250 Date: 14/05/2019 Nodes: 208 Sheet: 4 of 5 Printed by: Property Searches SEWER							CC Concrete Box Culverted
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MAR Masony, Random U Unspecified Address or Site Reference: Land at Walshaw Bury, OS sheet SD7812SE Number: Scale: 1:1250 Date: 14/05/2019 Nodes: 208 Sheet: 4 of 5 Printed by: Property Searches SEWER							
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3.9	610			VC	35,8469	1 in 43	
.11	300			VC	68,11755	1 in 296	
22	525			CO	28,97422	1 in 27	
22	525			co	28.97422	1 in 27	
4.1	300			VC	104.4845	1 in 39	
.24	525			VC	33.60059	1 in 59	
	100			VC	4,397091		
.85	225			co	31,76476		
	100			VC	4,748457		
8.5	150			VC	14.81982 8.602325		
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25	225			VC	60.16644	1 in 57	
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5	225			VC	19.63043		
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.42	375			co	20,17738		
>	300			VC	36.81654		
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0	225			VC	44,28318		
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.14	225			VC	67,20119	1 in 56	
5	300			VC	9.71132		
2	225			VC	30.41381 62.79255		
	150			VC	31.56059		
5	225			VC	34,9285		
.41	700			cc	17.20465		
	100			VC	6.373189		
.34	300			VC	12,20656	1 in 58	
1,39	375			VC	17.69181	1 in 111	
2	225			VC	23,18482		
5	225			VC	23.18482		
	100			VC	14,38278		
	100			VC	14.38278		
7.5	375			VC	31,62278	1 in 158	
1	150			VC	29.52051	1 in 10	
	100			VC	5.069386		
80.8	375			CC	6.967808 34.1784		
.05	150			vc	38,62058	1 in 102	
.51	300			CC	14,76482	1 In 102	
	1200			co	38,3414	1 in 18	
13	750			co	46.48409	1 in 930	
1.2	225			PVC	17.03697	1 in 14	
0	225			VC	18.25481		
D	225			VC	18.25481		
	150			VC	15.01898		
	150			VC	15.01898		
.68	1200			co	104,4151		
D	225			VC	19,17797		
D	225			VC	19.17797		
	100			PVC PVC	6.388364		
D	225			VC	6.388364		
b	225			VC	24.47787 24.47787		
0	100			VC	2,976499		
	100			VC	2.976499		
5.67	525			co	39.40817	1 in 69	
3.85	225			VC	7.211102		
.94	225			PVC	24.20853	1 in 14	
	0			UN	0.3709463		
0.28	225			VC	53,00943	1 in 24	
0,28	225			VC	53,00943	1 in 24	

LEGEND

Abandoned Foul Surface Water Combined Public Server Private Server Section 104 Raing Main Studge Main Overflow Water Course Highway Drain

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Change of Characteristi

MANHOLE FUNCTION

FO	Foul
SW	Surface Water
со	Combined
OV	Overflow

SEWER SHAPE

CI	Circular	TR	Trapezoidal
EG	Egg	AR	Arch
ov	Oval	BA	Barrel
FT	Flat Top	ю	HorseShoe
RE	Reclangular	UN	Unspecified
SQ	Square		

SEWER MATERI

AC	Asbestos Cement
BR	Brick
PE	Polyethylene
RP	Reinforced Plastic Matrix
co	Concrete
CSB	Concrete Segment Bolted
CSU	Concrete Segment Unbolted
CC	Concrete Box Culverted
PSC	Plastic / Steel Composite
GRC	Glass Reinforecd Plastic
DI	Ductile Iron
PVC	Polyvinyl Chloride
CI	Cast Iron
SI	Spun Iron
ST	Steel
VC	Vitrified Clay
PP	Polypropylane
PF	Pitch Fibre
MAC	Masonry, Coursed
MAR	Masonry, Random
U	Unspecified

Land at Walshaw Bury,

 OS sheet
 SD7811NE

 Number:
 Scale:

 1:1250
 Date:

 Nodes:
 268

 Sheet:
 5 of 5

Date: 14/05/2019

oping life flow smoothly

Printed by: Property Searches

SEWER RECORDS

Reece McGuinness

From:	Wastewater Developer Services < WastewaterDeveloperServices@uuplc.co.uk>
Sent:	06 January 2020 11:17
То:	Reece McGuinness; Wastewater Developer Services
Subject:	RE: File Transfer: 4072 - Land at Walshaw Bury Pre-Development Enquiry -
	4200029693

Dear Customer

We have carried out an assessment of your application which is based on the information provided; this pre development advice will be valid for 12 months

Foul will be allowed to drain to the public combined sewer network at an unrestricted rate. The connection(s) to the public sewer can be at a point(s) convenient to yourself

Surface water from this site should drain to either soak away/infiltration system or directly to watercourse. Discharge rates and consents must be discussed and agreed with all interested parties.

If you require any further guidance please follow http://www.unitedutilities.com/builders-developers.aspx

If you need a make further enquiry relating to this matter please send your enquiry to <u>WastewaterDeveloperServices@uuplc.co.uk</u> Please quote your DEXXXX/42XXXXX/GEXXXX reference number

Please Note:- enquiries sent to any other United Utilities e-mail address will be deleted.

Connection Application

Although we may discuss and agree discharge points & rates in principle, please be aware that you will have to apply for a formal sewer connection. This is so that we can assess the method of construction, Health & Safety requirements and to ultimatley inspect the connection when it is made. Details of the application process and the form itself can be obtained from our website by following the link below

http://www.unitedutilities.com/connecting-public-sewer.aspx

Please be aware that on site drainage must be designed in accordance with Building Regulations, National Planning Policy, Planning Conditions and local flood authority guidelines, we would recommend that you laise and make suitable agreements with the relevant statutory bodies.



Neil O'Brien Wastewater Pre-Development Engineer Great Manchester Area Developer Services & Metering Customer Services T: 01925 679410 unitedutilities.com

From: Reece McGuinness [mailto:reece.mcguinness@rocconsulting.com]
Sent: 24 December 2019 12:36
To: Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>
Subject: File Transfer: 4072 - Land at Walshaw Bury Pre-Development Enquiry (Redrow Homes) - Land at Walshaw, Bury

IMPORTANT: Click a link below to access files associated with this transmittal that came in through the RoC Consulting Info Exchange web site. <u>Download all associated files</u>

Additional links: **Reply to All** Project Land at Walshaw, Bury Name: Project 4072 Number: From: **Reece McGuinness** WastewaterDeveloperServices@uuplc.co.uk To: CC: David Eato; Paul White (RoC Consulting) Subject: 4072 - Land at Walshaw Bury Pre-Development Enquiry (Redrow Homes) Sent via: Info Exchange **Expiration** 1/23/2020 Date: Remarks: Dear Sir/Madam, Please find attached pre-development enquiry for the above site. Also attached is a plan indicating layout and an ownership plan. This pre-development enquiry relates to the Redrow Homes land. **Kind regards Reece McGuinness Strategic Land Graduate Engineer** T 0161 214 5390 www.rocconsulting.com



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behalf of RoC Consulting. Registered Address: RoC Consulting, Commercial Wharf, 6 Commercial Street, Manchester M15 4PZ. Company Registration No: 3126673

Transferred Files

NAME	ТҮРЕ	DATE	TIME	SIZE
Transmittal - 00003.pdf	PDF File	12/24/2019	12:39	190
			PM	КВ
30860-MR-M-021-Block	JPEG Image	12/24/2019	12:05	6,446
Layout-A1_PT_1.2500-01-			PM	КВ
<u>01.jpg</u>				
<u>30860-MR-M-04-Land</u>	JPEG Image	12/24/2019	12:06	12,405
Ownership Plan-			PM	КВ
A1_PT_1.2500-01.jpg				
Redrow Pre Development	PDF File	12/24/2019	12:04	1,472
Enquiry.pdf			PM	КВ

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Kind regards Reece McGuinness Strategic Land Graduate Engineer T 0161 214 5390 www.rocconsultina.com



EMGateway3.uuplc.co.uk made the following annotations

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Reece McGuinness

From: Sent:	O'Brien, Neil <neil.o'brien@uuplc.co.uk> 06 January 2020 11:04</neil.o'brien@uuplc.co.uk>
То:	Reece McGuinness; Wastewater Developer Services
Subject:	RE: File Transfer: 4072 - Land at Walshaw Bury Pre-Development Enquiry (Vernon & Co) - Land at Walshaw, Bury - 4200029692

Dear Customer

We have carried out an assessment of your application which is based on the information provided; this pre development advice will be valid for 12 months

Foul will be allowed to drain to the public combined sewer network at an unrestricted rate. The connection(s) to the public sewer can be at a point(s) convenient to yourself

Surface water from this site should drain to either soak away/infiltration system or directly to watercourse. Discharge rates and consents must be discussed and agreed with all interested parties.

If you require any further guidance please follow http://www.unitedutilities.com/builders-developers.aspx

If you need a make further enquiry relating to this matter please send your enquiry to <u>WastewaterDeveloperServices@uuplc.co.uk</u> Please quote your DEXXXX/42XXXXX/GEXXXX reference number

Please Note:- enquiries sent to any other United Utilities e-mail address will be deleted.

Connection Application

Although we may discuss and agree discharge points & rates in principle, please be aware that you will have to apply for a formal sewer connection. This is so that we can assess the method of construction, Health & Safety requirements and to ultimatley inspect the connection when it is made. Details of the application process and the form itself can be obtained from our website by following the link below

http://www.unitedutilities.com/connecting-public-sewer.aspx

Please be aware that on site drainage must be designed in accordance with Building Regulations, National Planning Policy, Planning Conditions and local flood authority guidelines, we would recommend that you laise and make suitable agreements with the relevant statutory bodies.

From: Reece McGuinness [mailto:reece.mcguinness@rocconsulting.com]
Sent: 24 December 2019 12:35
To: Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>
Subject: File Transfer: 4072 - Land at Walshaw Bury Pre-Development Enquiry (Vernon & Co) - Land at Walshaw, Bury

IMPORTANT: Click a link below to access files associated with this transmittal that came in through the RoC Consulting Info Exchange web site. Download all associated files

Additional links:

Additional	iiiks.				
Reply to Al					
Project	Land at Walshaw, Bury				
Name:					
Project	4072				
Number:					
From:	Reece McGuinness				
То:	WastewaterDeveloperServices@uuplc.co.uk				
CC:	David Eato; Paul White (RoC Consulting)				
Subject:	4072 - Land at Walshaw Bury Pre-Development Enquiry (Vernon & Co)				
Sent via:	Info Exchange				
Expiration	1/23/2020				
Date:					
Remarks:	Dear Sir/Madam,				
	Please find attached pre-development enquiry for the above site.				
	Also attached is a plan indicating layout and an ownership plan. This pre-development				
	enquiry relates to the Vernon & Co land.				
	Kind regards Reece McGuinness				
	Strategic Land Graduate Engineer				
	T 0161 214 5390				
	www.rocconsulting.com				
	Complex challenges Made simple				

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Transferred Files

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			PM	KB
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Layout-A1_PT_1.2500-01-			PM	КВ
<u>01.jpg</u>				
30860-MR-M-04-Land	JPEG Image	12/24/2019	12:06	12,405
Ownership Plan-			PM	КВ
A1_PT_1.2500-01.jpg				
Vernon Pre Development	PDF File	12/24/2019	12:10	1,447
Enquiry.pdf			PM	КВ

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Kind regards Reece McGuinness Strategic Land Graduate Engineer T 0161 214 5390



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APPENDIX E – CATCHMENT AREA PLANS









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APPENDIX F – SURFACE WATER RUNOFF CALCULATIONS

ROC Consulting		Page 1
Commercial Wharf	Prim Sch SW GF Run Off	
6 Commercial Street	Bury Council	
Manchester M15 4PZ	Walshaw, Bury	Mirro
Date 03/01/2020 12:04	Designed by DAE	Drainage
File 4072-BC-PS-SW GF Run Of	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1123 Urban 0.000 Area (ha) 2.130 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural 17.9 QBAR Urban 17.9 Q1 year 15.6

Q1 year 15.6 Q30 years 30.3 Q100 years 37.2

ROC Consulting		Page 1
Commercial Wharf	Parcel A SW GF Runn Off	
6 Commercial Street	Himor	
Manchester M15 4PZ	Walshaw, Bury	Mirm
Date 07/01/2020 14:05	Designed by DAE	Drainage
File 4072-Hi-A-SW GF Run Off	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1137 Urban 0.000 Area (ha) 3.650 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural 31.1 QBAR Urban 31.1 Q1 year 27.0

Q1 year 27.0 Q30 years 52.7 Q100 years 64.7

ROC Consulting		Page 1
Commercial Wharf	Parcel B SW GF Run Off	
6 Commercial Street	Himor	
Manchester M15 4PZ	Walshaw, Bury	Micro
Date 07/01/2020 14:09	Designed by DAE	Drainage
File 4072-Hi-B-SW GF Run Off	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1137 Urban 0.000 Area (ha) 2.950 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural 25.1 QBAR Urban 25.1 Q1 year 21.9

Q1 year 21.9 Q30 years 42.6 Q100 years 52.3

ROC Consulting		Page 1
Commercial Wharf	Parcel C SW GF Run Off	
6 Commercial Street	Himor	
Manchester M15 4PZ	Walshaw, Bury	Mirro
Date 07/01/2020 14:13	Designed by DAE	Drainage
File 4072-Hi-C-SW GF Run Off	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1137 Urban 0.000 Area (ha) 0.620 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural	5.3
QBAR Urban	5.3
Q1 year	4.6
Q1 year	4.6
Q30 years	9.0
Q100 years	11.0

ROC Consulting		Page 1
Commercial Wharf	Parcel D SW GF Run Off	
6 Commercial Street	Himor	
Manchester M15 4PZ	Walshaw, Bury	Mirro
Date 07/01/2020 14:15	Designed by DAE	Drainage
File 4072-HI-D-SW GF RUN OFF	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1137 Urban 0.000 Area (ha) 0.320 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural QBAR Urban	
Q1 year	2.4
Q1 year Q30 years Q100 years	

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ROC Consulting		Page 1
Commercial Wharf	Road R1 SW GF Run Off	
6 Commercial Street	Himor	
Manchester M15 4PZ	Walshaw, Bury	Mirrn
Date 03/01/2020 09:06	Designed by 03.01.2020	Drainage
File 4072-Hi-R1-SW GF Run Of	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1137 Urban 0.000 Area (ha) 0.440 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural QBAR Urban		
Q1 year	3.3	
Q1 year Q30 years Q100 years	6.4	
ROC Consulting		Page 1
------------------------------	------------------------	----------
Commercial Wharf	Parcel A SW GF Run Off	
6 Commercial Street	Redrow Homes	
Manchester M15 4PZ	Walshaw, Bury	Micro
Date 02/01/2020 15:33	Designed by DAE	Drainage
File 4072-RED-A-SW GF RUN OF	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1109 Urban 0.000 Area (ha) 7.480 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural QBAR Urban	61.9 61.9
Q1 year	53.8
Q1 year	53.8
Q30 years	104.9
Q100 years	128.7

ROC Consulting		Page 1
Commercial Wharf	Parcel B SW GF Run Off	
6 Commercial Street	Redrow Homes	
Manchester M15 4PZ	Walshaw, Bury	Mirro
Date 02/01/2020 15:14	Designed by DAE	Drainage
File 4072-RED-B-SW GF RUN OF	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1109 Urban 0.000 Area (ha) 2.000 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural 16.5 QBAR Urban 16.5 Q1 year 14.4

Q1 year 14.4 Q30 years 28.0 Q100 years 34.4

ROC Consulting		Page 1
Commercial Wharf	Parcel C SW GF Run Off	
6 Commercial Street	Redrow Homes	
Manchester M15 4PZ	Walshaw, Bury	Mirro
Date 02/01/2020 15:30	Designed by DAE	Drainage
File 4072-RED-C-SW GF RUN OF	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1109 Urban 0.000 Area (ha) 0.120 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural QBAR Urban	
Q1 year	0.9
Q1 year Q30 years Q100 years	1.7

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ROC Consulting		Page 1
Commercial Wharf	Road SW GF Run Off	
6 Commercial Street	Redrow Homes	
Manchester M15 4PZ	Walshaw, Bury	Micro
Date 02/01/2020 15:39	Designed by DAE	Desinado
File 4072-RED-R-SW GF RUN OF	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1109 Urban 0.000 Area (ha) 0.110 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural QBAR Urban	
Q1 year	0.8
Q1 year Q30 years Q100 years	1.5

ROC Consulting		Page 1
Commercial Wharf	Parcel A SW GF Run Off	
6 Commercial Street	VWH Land Part (Walshaw) Ltd	
Manchester M15 4PZ	Walshaw, Bury	Micro
Date 03/01/2020 12:22	Designed by DAE	Drainage
File 4072-VWH-A-SW GF RUN OF	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1123 Urban 0.000 Area (ha) 1.120 Soil 0.470 Region Number Region 10

Results 1/s

 QBAR Rural
 9.4

 QBAR Urban
 9.4

 Q1 year
 8.2

 Q1 year
 8.2

 Q30 years
 15.9

 Q100 years
 19.6

ROC Consulting		Page 1
Commercial Wharf	Parcel B SW GF Run Off	
6 Commercial Street	VWH Land Part (Walshaw) Ltd	
Manchester M15 4PZ	Walshaw, Bury	Micro
Date 03/01/2020 12:59	Designed by DAE	Drainage
File 4072-VWH-B-SW GF Run Of	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1123 Urban 0.000 Area (ha) 3.960 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural 33.2 QBAR Urban 33.2 Q1 year 28.9

Q1 year 28.9 Q30 years 56.4 Q100 years 69.1

ROC Consulting		Page 1
Commercial Wharf	Parcel C SW GF Run Off	
6 Commercial Street	VWH Land Part (Walshaw) Ltd	
Manchester M15 4PZ	Walshaw, Bury	Mirro
Date 03/01/2020 13:05	Designed by DAE	Drainage
File 4072-VWH-C-SW GF Run Of	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1123 Urban 0.000 Area (ha) 2.200 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural 18.5 QBAR Urban 18.5 Q1 year 16.1

Q1 year 16.1 Q30 years 31.3 Q100 years 38.4

ROC Consulting		Page 1
Commercial Wharf	Parcel D SW GF Run Off	
6 Commercial Street	VWH Land Part (Walshaw) Ltd	
Manchester M15 4PZ	Walshaw, Bury	Mirro
Date 03/01/2020 13:09	Designed by DAE	Drainage
File 4072-VWH-D-SW GF Run Of	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1123 Urban 0.000 Area (ha) 0.720 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural	6.0
QBAR Urban	6.0
Q1 year	5.3
Q1 year	5.3
Q30 years	10.2
Q100 years	12.6

ROC Consulting		Page 1
Commercial Wharf	Parcel E SW GF Run Off	
6 Commercial Street	VWH Land Part (Walshaw) Ltd	
Manchester M15 4PZ	Walshaw, Bury	Micro
Date 03/01/2020 13:13	Designed by DAE	Drainage
File 4072-VWH-E-SW GF Run Of	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1123 Urban 0.000 Area (ha) 1.800 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural 15.1 QBAR Urban 15.1 Q1 year 13.1 Q1 year 13.1

Q1 year 13.1 Q30 years 25.6 Q100 years 31.4

ROC Consulting		Page 1
Commercial Wharf	Parcel F SW GF Run Off	
6 Commercial Street	VWH Land Part (Walshaw) Ltd	
Manchester M15 4PZ	Walshaw, Bury	Mirro
Date 03/01/2020 13:17	Designed by DAE	Drainage
File 4072-VWH-F-SW GF Run Of	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1123 Urban 0.000 Area (ha) 0.240 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural QBAR Urban	
Q1 year	1.8
Q1 year Q30 years Q100 years	3.4

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ROC Consulting		Page 1
Commercial Wharf	Parcel G SW GF Run Off	
6 Commercial Street	VWH Land Part (Walshaw) Ltd	
Manchester M15 4PZ	Walshaw, Bury	Micro
Date 03/01/2020 13:23	Designed by DAE	Drainage
File 4072-VWH-G-SW GF Run Of	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1123 Urban 0.000 Area (ha) 3.620 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural 30.4 QBAR Urban 30.4 Q1 year 26.4

Q1 year 26.4 Q30 years 51.5 Q100 years 63.2

ROC Consulting		Page 1
Commercial Wharf	Parcel H SW GF Run Off	
6 Commercial Street	VWH Land Part (Walshaw) Ltd	
Manchester M15 4PZ	Walshaw, Bury	Mirro
Date 03/01/2020 13:39	Designed by DAE	Drainage
File 4072-VWH-H-SW GF Run Of	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1123 Urban 0.000 Area (ha) 2.690 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural 22.6 QBAR Urban 22.6 Q1 year 19.6

Q1 year 19.6 Q30 years 38.3 Q100 years 47.0

ROC Consulting		Page 1
Commercial Wharf	Parcel I SW GF Run Off	
6 Commercial Street	VWH Land Part (Walshaw) Ltd	
Manchester M15 4PZ	Walshaw, Bury	Mirro
Date 03/01/2020 13:48	Designed by DAE	Drainage
File 4072-VWH-I-SW GF Run Of	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1123 Urban 0.000 Area (ha) 2.630 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural 22.1 QBAR Urban 22.1 Q1 year 19.2 Q1 year 19.2 Q30 years 37.4 Q100 years 45.9

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ROC Consulting		Page 1
Commercial Wharf	Spine Road 1 SW GF Run Off	
6 Commercial Street	VWH Land Part (Walshaw) Ltd	
Manchester M15 4PZ	Walshaw, Bury	Mirro
Date 03/01/2020 13:52	Designed by DAE	Drainage
File 4072-VWH-R1-SW GF Run O	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1123 Urban 0.000 Area (ha) 0.470 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural QBAR Urban	
Q1 year	3.4
Q1 year Q30 years Q100 years	6.7

ROC Consulting		Page 1
Commercial Wharf	Spine Road 2 SW GF Run Off	
6 Commercial Street	VWH Land Part (Walshaw) Ltd	
Manchester M15 4PZ	Walshaw, Bury	Mirro
Date 03/01/2020 13:55	Designed by DAE	Drainage
File 4072-VWH-R2-SW GF Run O	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1123 Urban 0.000 Area (ha) 0.260 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural QBAR Urban	
Q1 year	1.9
Q1 year Q30 years Q100 years	3.7

ROC Consulting		Page 1
Commercial Wharf	Spine Road 3 SW GF Run Off	
6 Commercial Street	VWH Land Part (Walshaw) Ltd	
Manchester M15 4PZ	Walshaw, Bury	Mirro
Date 03/01/2020 14:00	Designed by DAE	Drainane
File 4072-VWH-R3-SW GF Run O	Checked by PAW	Diamage
XP Solutions	Source Control 2019.1	

Input

Return Period (years) 1 SAAR (mm) 1123 Urban 0.000 Area (ha) 0.630 Soil 0.470 Region Number Region 10

Results 1/s

QBAR Rural 5.3 QBAR Urban 5.3 Ql year 4.6 Ql year 4.6 Q30 years 9.0 Q100 years 11.0



COMPLEX CHALLENGES ... MADE SIMPLE

APPENDIX G – SURFACE WATER ATTENUATION CALCULATIONS

4072 Primary School, Bury Council, Walshaw, Bury Quick Storage Estimate Calculation 03.01.2020

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 1.28Ha, Qmax= 18.2 litres/sec

	Variables						
licro	FSR Rainfall		~	Cv (Summer)		0.750	
rainage	Return Period	l (years)	30	Cv (Winter)		0.840	
Variables	Region	England and	Wales 🗸	Impermeable Area (h	a)	1.280	
Results	Map	M5-60 (mm)	20.000	Maximum Allowable (Discharge (1/s)	18.2	
	. Local and	Ratio R	0.300	Infiltration Coefficient	(m/hr)	0.00000	8
Design	-		·	Safety Factor		2.0	
Overview 2D				Climate Change (%)		0	
Overview 3D						NO.	
Vt							
				Analyse	OK	Cancel	Help
Quick Storage	Estimate	Enter Maximu	ım Allowable Disc	charge between 0.0 and	999999.0		
Quick Storage Licro rainage	Results Global Varia of between	ables require 292 m³ and 4	approximate s 486 m³.	torage			
licro	Results Global Varia of between	ables require 292 m³ and 4	approximate s 486 m³.				
Aicro Irainage	Results Global Varia of between	ables require 292 m³ and 4	approximate s 486 m³.	torage			
licro rainage Variables	Results Global Varia of between	ables require 292 m³ and 4	approximate s 486 m³.	torage			
licro rainage Variables Results	Results Global Varia of between	ables require 292 m³ and 4	approximate s 486 m³.	torage			
Variables Results Design	Results Global Varia of between	ables require 292 m³ and 4	approximate s 486 m³.	torage			
Variables Results Design	Results Global Varia of between	ables require 292 m³ and 4	approximate s 486 m³.	torage			
Variables Results Design Dverview 2D Dverview 3D	Results Global Varia of between	ables require 292 m³ and 4	approximate s 486 m³.	torage			Help

NB: Approximate attenuation volume taken as 486m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 1.28Ha,</u> <u>Qmax= 22.3 litres/sec</u>

🕖 Quick Storage	Estimate		
	Variables		
Micro Drainage	FSR Rainfall 🗸	Cv (Summer)	0.750
Diamage	Return Period (years) 100	Cv (Winter)	0.840
Variables	Region England and Wales V	Impermeable Area <mark>(</mark> ha)	1.280
Results	Map M5-60 (mm) 20.000	Maximum Allowable Discharge (I/s)	22.3
Design	Ratio R 0.300	Infiltration Coefficient (m/hr)	0.00000
Overview 2D		Safety Factor	2.0
		Climate Change (%)	40
Overview 3D			
Vt			
		Analyse OK	Cancel Help
	Enter Maximum Allowable Diec	harge between 0.0 and 999999.0	
	Litter Maximum Alowable blac	halge between 0.0 and 555555.0	
🕖 Quick Storage	: Estimate		
	Results		
Micro	Global Variables require approximate a of between 633 m ³ and 986 m ³ .	storage	
Drainage	These values are estimates only and s	should not be used for design numo	60¢
Variables	These values are estimates only and a	aloud not be used for design purpo	aua.
Results			
Design			
Overview 2D			
	-		
Overview 3D			
Vt			
		Analyse OK	Cancel Help
	Enter Maximum Allowable Dier	charge between 0.0 and 999999.0	

NB: Approximate attenuation volume taken as 986m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.

4072 Parcel A Himor, Walshaw, Bury Quick Storage Estimate Calculation 07.01.2020

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 2.19Ha, Qmax= 31.6 litres/sec

No.	Variables				
Micro	FSR Rainfall		~	Cv (Summer)	0.750
Drainage	Return Period	(years)	30	Cv (Winter)	0.840
Variables	Region	England and	i Wales 🗸 🗸	Impermeable Area (ha)	2.190
Results	Мар	M5-60 (mm)	20.000	Maximum Allowable Discharge (1/s)	31.6
Design	-	Ratio R	0.300	Infiltration Coefficient (m/hr)	0.00000
Overview 2D	-			Safety Factor	2.0
	-			Climate Change (%)	0
Overview 3D	-				
Vt					
				Analyse OK	Cancel Help
		5.0	~ . ~	1	
		Ent	er Climate Change	e between -100 and 600	
🖊 Quick Storage	Estimate				
Quick Storage	Estimate Results				
Quick Storage	Results	ibles require 497 m³ and	approximate s 827 m³.	torage	
Micro Drainage	Results Global Varia of between	497 m ³ and	827 m ³ .	torage hould not be used for design purpo	
Micro Drainage Variables	Results Global Varia of between	497 m ³ and	827 m ³ .		
Micro Drainage	Results Global Varia of between	497 m ³ and	827 m ³ .		
Micro Drainage Variables	Results Global Varia of between	497 m ³ and	827 m ³ .		
Micco Drainage Variables Results	Results Global Varia of between	497 m ³ and	827 m ³ .		
Micco Drainage Variables Results Design	Results Global Varia of between	497 m ³ and	827 m ³ .		
Micro Drainage Variables Results Design Overview 2D	Results Global Varia of between	497 m ³ and	827 m ³ .		
Micro Drainage Variables Results Design Overview 2D Overview 3D	Results Global Varia of between	497 m ³ and	827 m ³ .		
Micro Drainage Variables	Results Global Varia of between	497 m ³ and	827 m ³ .		

NB: Approximate attenuation volume taken as 827m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 2.19Ha,</u> <u>Q max= 38.8 litres/sec</u>

🕖 Quick Storage	Estimate		
	Variables		
Micro	FSR Rainfall	Cv (Summer)	0.750
Drainage	Return Period (years) 100	Cv (Winter)	0.840
Variables	Region England and Wales	Impermeable Area (ha)	2.190
Results	Map M5-60 (mm) 20.000	Maximum Allowable Discharge (1/s)	38.8
Design	Ratio R 0.300	Infiltration Coefficient (m/hr)	0.00000
-		Safety Factor	2.0
Overview 2D		Climate Change (%)	40
Overview 3D			
Vt			
🗸 Quick Storage		ange between -100 and 600	- • •
	Results		
Micro Drainage	Global Variables require approximal of between 1076 m ³ and 1677 m ³ .	te storage	
Variables	These values are estimates only an	d should not be used for design purpos	es.
Results			
Design			
Overview 2D			
Overview 3D			
Vt			
Vt		Analyse OK	Cancel Help

NB: Approximate attenuation volume taken as 1677m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.

<u>4072 Parcel B Himor, Walshaw, Bury Quick Storage Estimate Calculation</u> <u>07.01.2020</u>

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 1.77Ha, Qmax= 25.6 litres/sec

💋 Quick Storage	Estimate			
	Variables			
Micro Drainage	FSR Rainfall	~	Cv (Summer)	0.750
Diamage	Return Period (years)	30	Cv (Winter)	0.840
Variables	Region England and	Wales 🗸	Impermeable Area (ha)	1.770
Results	Map M5-60 (mm)	20.000	Maximum Allowable Discharge (I/s)	25.6
Design	Ratio R	0.300	Infiltration Coefficient (m/hr)	0.00000
Overview 2D	-		Safety Factor	2.0
Overview 3D			Climate Change (%)	0
Vt	-			
			Analyse OK	Cancel Help
	Enter Maximu	m Allowable Discl	harge between 0.0 and 999999.0	
V O ist Orange	Patricela			
🖌 Quick Storage	ľ.			
	Results			
Øuick Storage Micro Drainage	ľ.	approximate s 667 m ³ .	torage	
Micro Drainage	Results Global Variables require of between 401 m ³ and	667 m ³ .	torage hould not be used for design purp	
Micro	Results Global Variables require of between 401 m ³ and	667 m ³ .		
Micro Drainage	Results Global Variables require of between 401 m ³ and	667 m ³ .		
Micro Drainage Variables	Results Global Variables require of between 401 m ³ and	667 m ³ .		
Micro Drainage Variables Results	Results Global Variables require of between 401 m ³ and	667 m ³ .		
Micro Drainage Variables Results Design	Results Global Variables require of between 401 m ³ and	667 m ³ .		
Micro Drainage Variables Results Design Overview 2D	Results Global Variables require of between 401 m ³ and	667 m ³ .		
Micro Drainage Variables Results Design Overview 2D Overview 3D	Results Global Variables require of between 401 m ³ and	667 m ³ .		
Micro Drainage Variables Results Design Overview 2D Overview 3D	Results Global Variables require of between 401 m ³ and 0 These values are estimated	667 m ³ . Ites only and s	hould not be used for design purp	oses.

NB: Approximate attenuation volume taken as 667m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 1.77Ha,</u> <u>Qmax= 31.4 litres/sec</u>

🕖 Quick Storage	Estimate						
	Variables						
Micro	FSR Rainfall		~	Cv (Summer)	1	0.750	
Drainage	Return Period ((years)	100	Cv (Winter)		0.840	=
Variables	Region	England and	Wales 🗸	Impermeable Area (ha)		1.770	
Results	Map	M5-60 (mm)	20.000	Maximum Allowable Disch	arge (l/s)	31.4	
		Ratio R	0.300	Infiltration Coefficient (m/h	ır)	0.00000	
Design	r.			Safety Factor		2.0	
Overview 2D				Climate Change (%)		40	
Overview 3D							
Vt							
				Analyse	ОК	Cancel	Help
	1	Enter Maximu	m Allowable Discl	harge between 0.0 and 9999	999.0		
🕖 Quick Storage	Estimate					0	
	Results						
Micro		oles require	approximate s	torage			
Drainage	of between 8						
Variables	These values	s are estima	ates only and s	hould not be used for d	esign purpose	IS.	
Results							
Design							
Overview 2D							
Overview 3D							
Vt							
				Analyse	ОК	Cancel	Help

NB: Approximate attenuation volume taken as 1355m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.

4072 Parcel C Himor, Walshaw, Bury Quick Storage Estimate Calculation 07.01.2020

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 0.38Ha, Qmax= 5.4 litres/sec

· · · · · · · · · · · · · · · · · · ·					
	Variables				
Micro	FSR Rainfall	8	~	Cv (Summer)	0.750
Drainage	Return Period	l (years)	30	Cv (Winter)	0.840
Variables	Region	England and	Wales 🗸	Impermeable Area (ha)	0.380
Results	Map	M5-60 (mm)	20.000	Maximum Allowable Discharge (I/s)	5.4
Design		Ratio R	0.300	Infiltration Coefficient (m/hr)	0.00000
	-			Safety Factor	2.0
Overview 2D				Climate Change (%)	0
Overview 3D					
Vt					
				Analyse OK	Cancel Help
				, L, L.	
		Enter Maximu	ım Allowable Discl	harge between 0.0 and 999999.0	
Quick Storage	Estimate				
	Results				
Micro	Global Varia	bles require	approximate st 44 m³.	orage	
Drainage	of between a	8/m³ and 14			
				nould not be used for design purpo	ses.
Variables					ses.
					ses.
Variables					ses.
Variables Results					Ses.
Variables Results Design					ses.
Variables Results Design Overview 2D					ses.
Variables Results Design Overview 2D Overview 3D					Cancel Help

NB: Approximate attenuation volume taken as 144m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 0.38Ha,</u> <u>Qmax= 6.6 litres/sec</u>

💋 Quick Storage	Estimate			
	Variables			
Micro	FSR Rainfall	~	Cv (Summer)	0.750
Drainage	Return Period (years)	100	Cv (Winter)	0.840
Variables	Region England and	Wales 🗸	Impermeable Area (ha)	0.380
Results	Map M5-60 (mm)	20.000	Maximum Allowable Discharge (1/s)	6.6
Design	Ratio R	0.300	Infiltration Coefficient (m/hr)	0.00000
Overview 2D			Safety Factor	2.0
			Climate Change (%)	40
Overview 3D				
Vt				
			Analyse OK	Cancel Help
	Enter Maximu	m Allowable Disc	harge between 0.0 and 999999.0	
🕖 Quick Storage	Estimate			
🖉 Quick Storage	Estimate Results			
Micro	Results Global Variables require	e approximate :	storage	
	Results Global Variables require of between 188 m ³ and	293 m ³ .		
Micro	Results Global Variables require of between 188 m ³ and	293 m ³ .	storage should not be used for design pu	
Micro Drainage	Results Global Variables require of between 188 m ³ and	293 m ³ .		
Micro Drainage Variables	Results Global Variables require of between 188 m ³ and	293 m ³ .		
Micro Drainage Variables Results	Results Global Variables require of between 188 m ³ and	293 m ³ .		
Variables Results Design	Results Global Variables require of between 188 m ³ and	293 m ³ .		
Variables Variables Results Design Overview 2D	Results Global Variables require of between 188 m ³ and	293 m ³ .		
Variables Variables Results Design Overview 2D Overview 3D	Results Global Variables require of between 188 m ³ and	293 m ³ .		
Variables Variables Results Design Overview 2D Overview 3D	Results Global Variables require of between 188 m ³ and These values are estimated	293 m³. ates only and s	should not be used for design pu	rposes.

NB: Approximate attenuation volume taken as 293m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.

4072 Parcel D Himor, Walshaw, Bury Quick Storage Estimate Calculation 07.01.2020

(MicroDrainage Source Control 2019.1)

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 0.20Ha,</u> <u>Qmax= 5 litres/sec</u>

	Variables							
Micro Drainage	FSR Rainfall			~	Cv (Summer)		0.750	
stelltrage	Return Period	(vears)	100		Cv (Winter)		0.840	
Variables	Region	England and	Wales	~	Impermeable Area (ha)		0.200	
Results	Мар	M5-60 (mm)	20.000		Maximum Allowable Discharge (1/	s)	5.0	
Design		Ratio R	0.300		Infiltration Coefficient (m/hr)		0.00000	8
-	-				Safety Factor		2.0	
Overview 2D	-				Climate Change (%)		40	
Overview 3D								
Vt								
					Analyse OK		Cancel	Help

V Quick Storage	e Estimate	
	Results	
Micro Drainage	Global Variables require approximate storage of between 85 m ³ and 136 m ³ . These values are estimates only and should not be used for design purposes.	
Variables		
Results		
Design		
Overview 2D		
Overview 3D		
Vt		
	Analyse OK Cancel	Help
	Enter Ratio R between 0.050 and 0.500	

NB: Approximate attenuation volume taken as 136m³ subject to confirmation at detail design, maximum discharge restricted to minimum practical rates of 5 l/s in accordance with NPPF Guidelines.

4072 Parcel R1 Himor, Walshaw, Bury Quick Storage Estimate Calculation 03.01.2020

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 0.44Ha, Qmax= 6.4 litres/sec

	Variables				
Micro	FSR Rainfall	~	Cv (Summer)	0.750	
Drainage	Return Period (years)	30	Cv (Winter)	0.840	
Variables	Region England a	and Wales 🗸 🗸	Impermeable Area (ha)	0.440	
Results	Map M5-60 (mn	n) 20.000	Maximum Allowable Discharge (1/s)	6.4	
Design	Ratio R	0.300	Infiltration Coefficient (m/hr)	0.00000	8
Overview 2D			Safety Factor	2.0	
	-		Climate Change (%)	0	
Overview 3D	_				
Vt					
			Analyse OK	Cancel	Help

🕖 Quick Storage	e Estimate	• ×
	Results	
Micro Drainage	Global Variables require approximate storage of between 99 m³ and 166 m³.	
Variables	These values are estimates only and should not be used for design purposes.	
Results		
Design		
Overview 2D		
Overview 3D		
Vt		
	Analyse OK Cancel	Help
	Enter Climate Change between -100 and 600	

NB: Approximate attenuation volume taken as 166m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 0.44Ha,</u> <u>Qmax= 7.8 litres/sec</u>

	Variables					
Aicro Irainage	FSR Rainfall Return Period	the PREAM PROVIDENT AND	~	Cv (Summer) Cv (Winter)	0.750	
Variables	Region	England and	Wales ~	Impermeable Area (ha)	0.440	
Results	Мар	M5-60 (mm) Ratio R	20.000	Maximum Allowable Discharge (l/s) Infiltration Coefficient (m/hr)	7.8	8
Design Overview 2D				Safety Factor	2.0	
Dverview 3D	-			Climate Change (%)	40	
Vt						
				Analyse OK	Cancel	Help

🕖 Quick Storage	Estimate	
	Results	
Micro Drainage	Global Variables require approximate storage of between 216 m ³ and 337 m ³ .	
Variables	These values are estimates only and should not be used for design purposes.	
Results		
Design		
Overview 2D		
Overview 3D		
Vt		
	Analyse OK Cance	Help
	Enter Climate Change between -100 and 600	

NB: Approximate attenuation volume taken as 337m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.

<u>4072 Parcel A Redrow Homes, Walshaw, Bury Quick Storage Estimate</u> <u>Calculation 02.01.2020</u>

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 4.49Ha, Qmax= 63.0 litres/sec

	Variables						
Aicro	FSR Rainfall		~	Cv (Summer)		0.750	1
Irainage	Return Period	(years)	30	Cv (Winter)		0.840	-
Variables	Region	England and	i Wales 🗸	Impermeable Area (ha)		4.490	
Results	Мар	M5-60 (mm)	20.000	Maximum Allowable Disch	arge (1/s)	63.0	
Annen area		Ratio R	0.300	Infiltration Coefficient (m/h	r)	0.00000	8
Design				Safety Factor		2.0	
Overview 2D				Climate Change (%)		0	
Overview 3D				canato calango (%)		L	
Vt							
					- C14	-	
Quick Storage	Estimate Results	Enter Maximu	ım Allowable Disc	Analyse harge between 0.0 and 9999	OK	Cancel	Help
licro	Results Global Varia of between	bles require 1030 m³ and	approximate s 1 1713 m ³ .	harge between 0.0 and 9999	99.0		
licro	Results Global Varia of between	bles require 1030 m³ and	approximate s 1 1713 m ³ .	harge between 0.0 and 9999	99.0		
licro rainage	Results Global Varia of between	bles require 1030 m³ and	approximate s 1 1713 m ³ .	harge between 0.0 and 9999	99.0		
licro rainage Variables	Results Global Varia of between	bles require 1030 m³ and	approximate s 1 1713 m ³ .	harge between 0.0 and 9999	99.0		
Variables Results Design	Results Global Varia of between	bles require 1030 m³ and	approximate s 1 1713 m ³ .	harge between 0.0 and 9999	99.0		
Results	Results Global Varia of between	bles require 1030 m³ and	approximate s 1 1713 m ³ .	harge between 0.0 and 9999	99.0		
Variables Results Design	Results Global Varia of between	bles require 1030 m³ and	approximate s 1 1713 m ³ .	harge between 0.0 and 9999	99.0		

NB: Approximate attenuation volume taken as 1713m³ upper.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 4.49Ha,</u> <u>Qmax= 77.2 litres/sec</u>

	Variables							
Aicro	FSR Rainfall			~ (Cv (Summer)		0.750	
lrainage	Return Period	(years)	100	- i	Cv (Winter)		0.840	
Variables	Region	England and	i Wales	~ I	Impermeable Area (ha)		4.490	
Results	Мар	M5-60 (mm)	20.000		Maximum Allowable Dis	scharge (l/s)	77.2	
1.0000000		Ratio R	0.300	1	Infiltration Coefficient (r	n/hr)	0.00000	8
Design				:	Safety Factor		2.0	
Overview 2D				(Climate Change (%)		40	
Overview 3D								
Vt								
	Estimate	Enter Maximu	ım Allowable	Dischar	Analyse gebetween 0.0 and 9	OK 999999.0	Cancel	Help
		Enter Maximu	ım Allowable	Dischar				
Quick Storage	Results Global Varia of between 2	bles require 2230 m³ and	approxima 1 3475 m ³ .	ate stor	rge between 0.0 and 9	99999.0		Help
Quick Storage	Results Global Varia of between 2	bles require 2230 m³ and	approxima 1 3475 m ³ .	ate stor	rge between 0.0 and 9	99999.0		
Quick Storage Licro rainage	Results Global Varia of between 2	bles require 2230 m³ and	approxima 1 3475 m ³ .	ate stor	rge between 0.0 and 9	99999.0		
Quick Storage licco rainage Variables	Results Global Varia of between 2	bles require 2230 m³ and	approxima 1 3475 m ³ .	ate stor	rge between 0.0 and 9	99999.0		
Quick Storage Licro rainage Variables Results Design	Results Global Varia of between 2	bles require 2230 m³ and	approxima 1 3475 m ³ .	ate stor	rge between 0.0 and 9	99999.0		
Quick Storage Licro rainage Variables Results	Results Global Varia of between 2	bles require 2230 m³ and	approxima 1 3475 m ³ .	ate stor	rge between 0.0 and 9	99999.0		

NB: Approximate attenuation volume taken as 3475m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.

Enter Maximum Allowable Discharge between 0.0 and 999999.0

<u>4072 Parcel B Redrow Homes, Walshaw, Bury Quick Storage Estimate</u> <u>Calculation 02.01.2020</u>

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 1.20Ha, Qmax= 16.8 litres/sec

	Variables		
Micro Drainage	FSR Rainfall 🗸 🗸	Cv (Summer)	0.750
brainage	Return Period (years) 30	Cv (Winter)	0.840
Variables	Region England and Wales 🗸	Impermeable Area (ha)	1.200
Results	Map M5-60 (mm) 20.000	Maximum Allowable Discharge (I/s)	16.8
Design	Ratio R 0.300	Infiltration Coefficient (m/hr)	0.00000
-	-	Safety Factor	2.0
Overview 2D	-	Climate Change (%)	0
Overview 3D	_		
Vt			
		Analyse OK	Cancel Help

🕖 Quick Storage	Estimate	
Micro Drainage	Results Global Variables require approximate storage of between 276 m ³ and 458 m ³ . The sector of	
Variables	These values are estimates only and should not be used for design purposes.	
Results		
Design		
Overview 2D		
Overview 3D		
Vt		
	Analyse OK Cance	Help
	Enter Climate Change between -100 and 600	

NB: Approximate attenuation volume taken as 458m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 1.20Ha,</u> <u>Qmax= 21.8 litres/sec</u>

	Variables						
licro Irainage	FSR Rainfal	I	~	Cv (Summer)		0.750	
romoge	Return Perio	d (years)	100	Cv (Winter)		0.840	
Variables	Region	England and	i Wales 🗸 🗸	Impermeable Area (ha)	1.200	
Results	Мар	M5-60 (mm)	20.000	Maximum Allowable D	ischarge (l/s)	21.8	
Design		Ratio R	0.300	Infiltration Coefficient	(m/hr)	0.00000	6
Overview 2D				Safety Factor		2.0	
				Climate Change (%)		40	
Overview 3D							
Vt							
				Analyse	OK	Cancel	Help

💋 Quick Storage	e Estimate
	Results
Micro Drainage	Global Variables require approximate storage of between 584 m ³ and 911 m ³ .
Variables	These values are estimates only and should not be used for design purposes.
Results	
Design	
Overview 2D	
Overview 3D	
Vt	
	Analyse OK Cancel Help
	Enter Maximum Allowable Discharge between 0.0 and 999999.0

NB: Approximate attenuation volume taken as 911m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.

<u>4072 Parcel C Redrow Homes, Walshaw, Bury Quick Storage Estimate</u> <u>Calculation 02.01.2020</u>

(MicroDrainage Source Control 2019.1)

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 0.08Ha,</u> <u>Qmax= 5 litres/sec</u>

	Variables					
Variables Results Design Overview 2D		gland and 60 (mm)	100 Wales ~ 20.000 0.300	Cv (Summer) Cv (Winter) Impermeable Area (ha) Maximum Allowable Discharge (l/s) Infiltration Coefficient (m/hr) Safety Factor Climate Change (%)	0.750 0.840 0.080 5.0 0.00000 2.0 40	
Overview 3D Vt				Analyse OK	Cancel	Help

🖉 Quick Storage	Estimate
Micro Drainage	Results Global Variables require approximate storage of between 21 m ³ and 38 m ³ .
Variables	These values are estimates only and should not be used for design purposes.
Results	
Design	
Overview 2D	
Overview 3D	
Vt	
	Analyse OK Cancel Help
	Enter Maximum Allowable Discharge between 0.0 and 999999.0

NB: Approximate attenuation volume taken as 38m³ subject to confirmation at detail design, maximum discharge restricted to minimum practical rates of 5 l/s in accordance with NPPF Guidelines.

<u>4072 Road Redrow Homes, Walshaw, Bury Quick Storage Estimate</u> <u>Calculation 02.01.2020</u>

(MicroDrainage Source Control 2019.1)

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 0.11Ha,</u> <u>Qmax= 5 litres/sec</u>

Vuick Storage	Estimate Variables FSR Rainfall Return Period (years) 100	Cv (Summer) Cv (Winter)	0.750
Variables	Region England and Wales ~	Impermeable Area (ha)	0.110
Results	Map M5-60 (mm) 20.000	Maximum Allowable Discharge (I/s)	5.0
Design	Ratio R 0.300	Infiltration Coefficient (m/hr) Safety Factor	2.0
Overview 2D		Climate Change (%)	40
Overview 3D			20
Vt			
	·	Analyse OK	Cancel Help
	Enter Area betwee	en 0.000 and 999.999	

💋 Quick Storage	Estimate	×
	Results	
Micro Drainage	Global Variables require approximate storage of between 35 m ³ and 60 m ³ . These values are estimates only and should not be used for design purposes.	
Variables		
Results		
Design		
Overview 2D		
Overview 3D		
Vt		
	Analyse OK Cancel Help	
	Enter Area between 0.000 and 999.999	

NB: Approximate attenuation volume taken as 60m³subject to confirmation at detail design, maximum discharge restricted to minimum practical rates of 5 l/s in accordance with NPPF Guidelines.

<u>4072 Parcel A VWH Land Partnership (Walshaw) Limited , Walshaw, Bury</u> <u>Quick Storage Estimate Calculation 03.01.2020</u>

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 0.67Ha, Qmax= 9.5 litres/sec

1	Variables					
Aicro	FSR Rainfal	l	~	Cv (Summer)	0.750	
Irainage	Return Period	d (years)	30	Cv (Winter)	0.840	_
Variables	Region	England and	d Wales 🗸 🗸	Impermeable Area (ha)	0.670	
Results	Мар	M5-60 (mm)	20.000	Maximum Allowable Discharge (1/s)	9.5	
Design		Ratio R	0.300	Infiltration Coefficient (m/hr)	0.00000	8
	-			Safety Factor	2.0	
Overview 2D	_			Climate Change (%)	0	
Overview 3D						
Vt						
	S2					
				Analyse OK	Cancel	Help
				Analyse OK	Cancel	Help
		Enter Maximu	um Allowable Dis	Analyse OK charge between 0.0 and 999999.0	Cancel	Help
		Enter Maximu	um Allowable Dis		Cancel	Help
Quick Storage	e Estimate	Enter Maximu	um Allowable Dis			Help
Quick Storage	Estimate Results	Enter Maximu	um Allowable Dis			
	Results Global Varia	ables require	e approximate	charge between 0.0 and 999999.0		
	Results Global Varia of between	ables require 153 m³ and	approximate 254 m³.	charge between 0.0 and 9999999.0		
	Results Global Varia of between	ables require 153 m³ and	approximate 254 m³.	charge between 0.0 and 999999.0		
dicro Vainage	Results Global Varia of between	ables require 153 m³ and	approximate 254 m³.	charge between 0.0 and 9999999.0		
licro rainage Variables	Results Global Varia of between	ables require 153 m³ and	approximate 254 m³.	charge between 0.0 and 9999999.0		
licro rainage Variables Results	Results Global Varia of between	ables require 153 m³ and	approximate 254 m³.	charge between 0.0 and 9999999.0		
Variables Results Design	Results Global Varia of between	ables require 153 m³ and	approximate 254 m³.	charge between 0.0 and 9999999.0		
Variables Results Design Overview 2D Overview 3D	Results Global Varia of between	ables require 153 m³ and	approximate 254 m³.	charge between 0.0 and 9999999.0		
Variables Results Design	Results Global Varia of between	ables require 153 m³ and	approximate 254 m³.	charge between 0.0 and 9999999.0		

NB: Approximate attenuation volume taken as 254m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 0.67Ha,</u> <u>Qmax= 11.8 litres/sec</u>

📝 Quick Storage	Estimate			
Micro Drainage	Variables			
	FSR Rainfall V		Cv (Summer)	0.750
	Return Period (years)	100	Cv (Winter)	0.840
Variables	Region England and	Wales 🗸	Impermeable Area (ha)	0.670
Results	Map M5-60 (mm)	20.000	Maximum Allowable Discharge (1/s)	11.8
Design	Ratio R	0.300	Infiltration Coefficient (m/hr)	0.00000
-	-		Safety Factor	2.0
Overview 2D	-		Climate Change (%)	40
Overview 3D				
Vt				
			Analyse OK	Cancel Help
Enter Maximum Allowable Discharge between 0.0 and 999999.0				
1/ Quick Storage Estimate				
	Results			
Micro Drainage	Global Variables require approximate storage			
	of between 330 m ³ and 514 m ³ .			
Variables	These values are estimation	ites only and s	hould not be used for design purpo	oses.
Results				
Design				
Overview 2D				
Overview 3D				
Vt				
			Analyse OK	Cancel Help
Enter Maximum Allowable Discharge between 0.0 and 999999.0				

NB: Approximate attenuation volume taken as 514m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.
<u>4072 Parcel B VWH Land Partnership (Walshaw) Limited , Walshaw, Bury</u> <u>Quick Storage Estimate Calculation 03.01.2020</u>

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 2.38Ha, Qmax= 33.8 litres/sec

-					
	Variables				
Micro Drainage	FSR Rainfall		~	Cv (Summer)	0.750
Dialitage	Return Period (year	(ar	30	Cv (Winter)	0.840
Variables	Region Eng	land and	Wales 🗸	Impermeable Area (ha)	0.670
Results	Map M5-	60 (mm)	20.000	Maximum Allowable Discharge (1/s)	9.5
Design	Rati	o R	0.300	Infiltration Coefficient (m/hr)	0.00000
				Safety Factor	2.0
Overview 2D				Climate Change (%)	0
Overview 3D					
Vt					
				Analyse OK	Cancel Help
	Ente	er Maximur	n Allowable Disc	harge between 0.0 and 999999.0	
Quick Storage	Estimate				
	Results				1
Micro Drainage	Global Variables of between 153	require m ³ and 2	approximate s 254 m³.	torage	
	These values an	e estima	tes only and s	hould not be used for design purpo	ses.
Variables					
Results					
Results Design					
Design					
Design Overview 2D					
Design Overview 2D Overview 3D				Analyse OK	Cancel Help

NB: Approximate attenuation volume taken as 254m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 2.38Ha,</u> <u>Qmax= 41.5 litres/sec</u>

📝 Quick Storage	Estimate			
	Variables			
Micro	FSR Rainfall	~	Cv (Summer)	0.750
Drainage	Return Period (years)	100	Cv (Winter)	0.840
Variables	Region England and	Wales 🗸	Impermeable Area (ha)	0.670
Results	Map M5-60 (mm)	20.000	Maximum Allowable Discharge (I/s)	11.8
Design	Ratio R	0.300	Infiltration Coefficient (m/hr)	0.00000
	-		Safety Factor	2.0
Overview 2D	-		Climate Change (%)	40
Overview 3D				
Vt				
			Analyse OK	Cancel Help
	Enter Maximu	m Allowable Disc	harge between 0.0 and 999999.0	
/ Quick Storage	Estimate			
	Results			
Micro	Global Variables require	approximate s	torage	
Drainage	of between 330 m ³ and		-	
Variables	These values are estimation of the stime of	ites only and s	hould not be used for design purpo	ses.
Results				
Design				
Overview 2D				
Overview 3D				
Vt				
			Analyse OK	Cancel Help
			- Analyse OK	Concerning)
	Enter Maximu	m Allowable Discł	harge between 0.0 and 999999.0	

NB: Approximate attenuation volume taken as 514m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.

<u>4072 Parcel C VWH Land Partnership (Walshaw) Limited , Walshaw, Bury</u> <u>Quick Storage Estimate Calculation 03.01.2020</u>

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 1.32Ha, Qmax= 18.8 litres/sec

📝 Quick Storage	Estimate		
	Variables		
Micro	FSR Rainfall ~	Cv (Summer)	0.750
Drainage	Return Period (years) 30	Cv (Winter)	0.840
Variables	Region England and Wales 🗸	Impermeable Area (ha)	1.320
Results	Map M5-60 (mm) 20.000	Maximum Allowable Discharge (I/s)	18.8
Design	Ratio R 0.300	Infiltration Coefficient (m/hr)	0.00000
Overview 2D	-	Safety Factor	2.0
	-	Climate Change (%)	0
Overview 3D	-		
Vt			
		Analyse OK	Cancel Help
	Enter Maximum Allowable Di	scharge between 0.0 and 999999.0	
🗸 Quick Storage	Estimate		
	Results		
Micro Drainage	Global Variables require approximate of between 301 m ³ and 501 m ³ .	storage	
	These values are estimates only and	should not be used for design purpo	ses.
Variables			
Results			
Design			
Overview 2D			
Overview 3D			
Vt			
		Analyse OK	Cancel Help
	Enter Maximum Allowable Dir	scharge between 0.0 and 999999.0	

NB: Approximate attenuation volume taken as 501m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 1.32Ha,</u> <u>Qmax= 23.0 itres/sec</u>

💋 Quick Storage	Estimate		
	Variables		
Micro	FSR Rainfall V	Cv (Summer)	0.750
Drainage	Return Period (years) 100	Cv (Winter)	0.840
Variables	Region England and Wales ~	Impermeable Area (ha)	1.320
Results	Map M5-60 (mm) 20.000	Maximum Allowable Discharge (1/s)	23.0
Design	Ratio R 0.300	Infiltration Coefficient (m/hr)	0.00000
Overview 2D		Safety Factor	2.0
		Climate Change (%)	40
Overview 3D			
Vt			
		Analyse OK	Cancel Help
💋 Quick Storage	e Estimate Results		
Micro Drainage	Global Variables require approximate of between 652 m ³ and 1017 m ³ .	storage	
	These values are estimates only and	should not be used for design purp	oses.
Variables			
Results			
Design	_		
Overview 2D			
Overview 3D			
Vt			
		Analyse OK	Cancel Help

NB: Approximate attenuation volume taken as 1017m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.

<u>4072 Parcel D VWH Land Partnership (Walshaw) Limited , Walshaw, Bury</u> <u>Quick Storage Estimate Calculation 03.01.2020</u>

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 0.43Ha, Qmax= 6.1 litres/sec

🗸 Quick Storage	Estimate			
	Variables			
Micro Drainage	FSR Rainfall	~	Cv (Summer)	0.750
Diamaye	Return Period (years)	30	Cv (Winter)	0.840
Variables	Region England and	Wales 🗸	Impermeable Area (ha)	0.430
Results	Map M5-60 (mm)	20.000	Maximum Allowable Discharge (1/s)	6.1
Design	Ratio R	0.300	Infiltration Coefficient (m/hr)	0.00000
	-		Safety Factor	2.0
Overview 2D			Climate Change (%)	0
Overview 3D				
Vt				
	1		Analyse OK	Cancel Help
	Enter Maximu	m Allowable Disc	harge between 0.0 and 999999.0	
	200			
Quick Storage	Estimate			
	Results			
Micro Drainage	Global Variables require of between 98 m ³ and 1	approximate s 63 m ³ .	torage	
	These values are estimation	ites only and s	hould not be used for design purpo	ses.
Variables				
Results				
Design				
Overview 2D				
	-			
Overview 3D	-			
Vt				
			Analyse OK	Cancel Help
	Ester H. J.	en Alleurekte Dr		
	Enter Maximu	m Miowable Disc	harge between 0.0 and 999999.0	

NB: Approximate attenuation volume taken as 163m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 0.43Ha,</u> <u>Qmax= 7.6 litres/sec</u>

and the second sec	Estimate			
	Variables			
Micro Drainage	FSR Rainfall	~	Cv (Summer)	0.750
Diamaye	Return Period (years)	100	Cv (Winter)	0.840
Variables	Region England and	Wales 🗸 🗸	Impermeable Area (ha)	0.430
Results	Map M5-60 (mm)	20.000	Maximum Allowable Discharge (I/s)	7.6
Design	Ratio R	0.300	Infiltration Coefficient (m/hr)	0.00000
Overview 2D			Safety Factor	2.0
			Climate Change (%)	40
Overview 3D				
Vt				
			Analyse OK	Cancel Help
🗸 Quick Storage	Estimate Results			
Micro Drainage	Global Variables require of between 211 m ³ and	330 m ³ .	storage should not be used for design pu	
Variables	. These values are estimated	ates only and a	siloulu not be used for design pu	iposes.
Results				
Results Design				
	-			
Design				
Design Overview 2D				
Design Overview 2D Overview 3D			Analyse OK	Cancel Help

NB: Approximate attenuation volume taken as 330m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.

<u>4072 Parcel E VWH Land Partnership (Walshaw) Limited , Walshaw, Bury</u> <u>Quick Storage Estimate Calculation 03.01.2020</u>

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 1.08Ha, Qmax= 15.4 litres/sec

🖉 Quick Storage	Estimate				
	Variables				
Micro	FSR Rainfall	,	~	Cv (Summer)	0.750
Drainage	Return Period	(years)	30	Cv (Winter)	0.840
Variables	Region	England and	Wales 🗸	Impermeable Area (ha)	1.080
Results	Мар	M5-60 (mm)	20.000	Maximum Allowable Discharge (1/s)	15.4
Design		Ratio R	0.300	Infiltration Coefficient (m/hr)	0.00000
				Safety Factor	2.0
Overview 2D				Climate Change (%)	0
Overview 3D					
Vt					
- Let				Analyse OK	Cancel Help
		Enter Maximu	ım Allowable Disc	harge between 0.0 and 999999.0	
🕖 Quick Storage	Estimate				
🖉 Quick Storage	Estimate Results				
Micro	Results Global Varia	bles require	approximate s	torage	
	Results Global Varia of between	246 m ³ and	409 m ³ .		
Micro	Results Global Varia of between	246 m ³ and	409 m ³ .	torage hould not be used for design purp	
Micro Drainage Variables	Results Global Varia of between	246 m ³ and	409 m ³ .		
Micro Drainage Variables Results	Results Global Varia of between	246 m ³ and	409 m ³ .		
Micro Drainage Variables Results Design	Results Global Varia of between	246 m ³ and	409 m ³ .		
Micro Drainage Variables Results	Results Global Varia of between	246 m ³ and	409 m ³ .		
Micro Drainage Variables Results Design	Results Global Varia of between	246 m ³ and	409 m ³ .		
Variables Results Design Overview 2D	Results Global Varia of between	246 m ³ and	409 m ³ .		
Micro Drainage Variables Results Design Overview 2D Overview 3D	Results Global Varia of between	246 m ³ and	409 m ³ .	hould not be used for design pur	poses.
Micro Drainage Variables Results Design Overview 2D Overview 3D	Results Global Varia of between	246 m ³ and	409 m ³ .		

NB: Approximate attenuation volume taken as 409m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 1.08Ha,</u> <u>Qmax= 18.8 litres/sec</u>

💋 Quick Storage	Estimate						
	Variables						
Micro	FSR Rainfall		~	Cv (Summer)		0.750	
Drainage	Return Period (year	rs)	100	Cv (Winter)		0.840	
Variables	Region Eng	land and	Wales 🗸	Impermeable Area (ha)		1.080	
Results	Map M5-6	60 (mm)	20.000	Maximum Allowable Disc	charge (I/s)	18.8	
Design	Ratio	o R	0.300	Infiltration Coefficient (m	/hr)	0.00000	8
Overview 2D	•			Safety Factor		2.0	
	-			Climate Change (%)		40	
Overview 3D							
Vt							
				Analyse	OK	Cancel	Help
	Ente	er Maximu	m Allowable Disc	harge between 0.0 and 99	9999.0		
1/ a : 1 a							
🖌 Quick Storage							
	Results			9			
Micro Drainage	Global Variables of between 534			torage			
	These values are	e estima	ates only and s	hould not be used for	design pur	poses.	
Variables							
Results							
Design							
Overview 2D							
Overview 3D							
Vt							
271 Sec.							
					01/		1
				Analyse	ОК	Cancel	Help

NB: Approximate attenuation volume taken as 832m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.

<u>4072 Parcel F VWH Land Partnership (Walshaw) Limited , Walshaw, Bury</u> <u>Quick Storage Estimate Calculation 03.01.2020</u>

(MicroDrainage Source Control 2019.1)

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 0.15Ha,</u> <u>Qmax= 5 litres/sec</u>

	Variables					
Aicro Drainage	FSR Rainfall Return Period	(years)	~	Cv (Summer) Cv (Winter)	0.750	
Variables	Region	England and	i Wales 🗸 🗸	Impermeable Area (ha)	0.150	
Results	Мар	M5-60 (mm)	20.000	Maximum Allowable Discharge (I/s)	5.0	
Design		Ratio R	0.300	Infiltration Coefficient (m/hr)	0.00000	
Overview 2D				Safety Factor	2.0	
Overview 2D				Climate Change (%)	40	
Vt						
				Analyse OK	Cancel	Help
		Enter Maximu	ım Allowable Dis	scharge between 0.0 and 999999.0		

	Results
licro Irainage	Global Variables require approximate storage of between 56 m ³ and 92 m ³ .
	These values are estimates only and should not be used for design purposes.
Variables	
Results	
Design	
Overview 2D	
Overview 3D	
Vt	

NB: Approximate attenuation volume taken as 92m³ subject to confirmation at detail design, maximum discharge restricted to minimum practical rates of 5 l/s in accordance with NPPF Guidelines.

<u>4072 Parcel G VWH Land Partnership (Walshaw) Limited , Walshaw, Bury</u> <u>Quick Storage Estimate Calculation 03.01.2020</u>

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 2.17Ha, Qmax= 30.9 litres/sec

	Variables				
Micro Drainage	FSR Rainfall		~	Cv (Summer)	0.750
biomage	Return Period	(years)	30	Cv (Winter)	0.840
Variables	Region	England and	Wales 🗸 🗸	Impermeable Area (ha)	2.170
Results	Мар	M5-60 (mm)	20.000	Maximum Allowable Discharge (//s) 30.9
Design		Ratio R	0.300	Infiltration Coefficient (m/hr)	0.00000
Overview 2D	-			Safety Factor	2.0
	_			Climate Change (%)	0
Overview 3D					
Vt					
				Analyse OK	Cancel Help
					and here and
		Enter Maximu	ım Allowable Disc	harge between 0.0 and 999999.0	
Quick Storag	e Estimate	Enter Maximu	ım Allowable Disc	harge between 0.0 and 999999.0	
		Enter Maximu	ım Allowable Disc	harge between 0.0 and 999999.0	
Quick Storag Micro Drainage	e Estimate Results	ibles require	e approximate :		
Micro	e Estimate Results Global Varia of between	ibles require 495 m³ and	e approximate : 823 m³.		
Micro	e Estimate Results Global Varia of between	ibles require 495 m³ and	e approximate : 823 m³.	storage	
Micro Drainage	e Estimate Results Global Varia of between	ibles require 495 m³ and	e approximate : 823 m³.	storage	
Micro Drainage Variables	e Estimate Results Global Varia of between	ibles require 495 m³ and	e approximate : 823 m³.	storage	
Micro Drainage Variables Results	e Estimate Results Global Varia of between	ibles require 495 m³ and	e approximate : 823 m³.	storage	
Micro Drainage Variables Results Design	e Estimate Results Global Varia of between	ibles require 495 m³ and	e approximate : 823 m³.	storage	
Micro Drainage Variables Results Design Overview 2D	e Estimate Results Global Varia of between	ibles require 495 m³ and	e approximate : 823 m³.	storage	
Micro Drainage Variables Results Design Overview 2D Overview 3D	e Estimate Results Global Varia of between	ibles require 495 m³ and	e approximate : 823 m³.	storage	

NB: Approximate attenuation volume taken as 823m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 2.17Ha,</u> <u>Qmax= 37.9 litres/sec</u>

🖉 Quick Storage	Estimate		
	Variables		18
Micro Drainage	FSR Rainfall	Cv (Summer)	0.750
Diamage	Return Period (years) 100	Cv (Winter)	0.840
Variables	Region England and Wales	V Impermeable Area (ha)	2.170
Results	Map M5-60 (mm) 20.000	Maximum Allowable Discharge (1/s)	37.9
Design	Ratio R 0.300	Infiltration Coefficient (m/hr)	0.00000
Overview 2D		Safety Factor	2.0
		Climate Change (%)	40
Overview 3D			
Vt			
		Analyse OK	Cancel Help
	Enter Maximum Allowa	ble Discharge between 0.0 and 999999.0	
	Lines meximum records	de blachaige betreen tit and 55555.5	
💋 Quick Storage	Estimate		
	Results		
	Clabel Veriables mention anomal	imate storage	
Micro	Global Variables require approxi		
Micro Drainage	of between 1072 m ³ and 1670 n	1 ³ .	
	of between 1072 m ³ and 1670 n	n ³ . y and should not be used for design purpo	uses.
Drainage	of between 1072 m ³ and 1670 n	1 ³ .	uses.
Variables Results	of between 1072 m ³ and 1670 n	1 ³ .	oses.
Variables Results Design	of between 1072 m ³ and 1670 n	1 ³ .	oses.
Variables Results Design Overview 2D	of between 1072 m ³ and 1670 n	1 ³ .	oses.
Variables Results Design	of between 1072 m ³ and 1670 n	1 ³ .	uses.
Variables Results Design Overview 2D	of between 1072 m ³ and 1670 n	1 ³ .	uses.
Variables Results Design Overview 2D Overview 3D	of between 1072 m ³ and 1670 n	1 ³ .	Dises.
Variables Variables Results Design Overview 2D Overview 3D	of between 1072 m ³ and 1670 m These values are estimates only	h ³ . y and should not be used for design purp	

NB: Approximate attenuation volume taken as 1670m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.

<u>4072 Parcel H VWH Land Partnership (Walshaw) Limited , Walshaw, Bury</u> <u>Quick Storage Estimate Calculation 03.01.2020</u>

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 1.62Ha, Qmax= 23.0 litres/sec

🕖 Quick Storage	Estimate		
	Variables		
Micro Drainage	FSR Rainfall 🗸	Cv (Summer)	0.750
oraniage	Return Period (years) 30	Cv (Winter)	0.840
Variables	Region England and Wales 🗸	Impermeable Area (ha)	1.620
Results	Map M5-60 (mm) 20.000	Maximum Allowable Discharge (I/s)	23.0
Design	Ratio R 0.300	Infiltration Coefficient (m/hr)	0.00000
Overview 2D		Safety Factor	2.0
Overview 3D		Climate Change (%)	0
Vt			
vt			
		Analyse OK	Cancel Help
	Enter Maximum Allowable Disc	charge between 0.0 and 999999.0	
V 0 : 1 0	F		
🖌 Quick Storage			
	Results	87 M.	
Micro Drainage	Global Variables require approximate of between 370 m ³ and 615 m ³ .	storage	
	These values are estimates only and	should not be used for design purpo	ses.
Variables			
Results			
Design			
Overview 2D			
Overview 3D			
Vt			
		Analyse OK	Cancel Help
	Enter Maximum Allowable Dise	charge between 0.0 and 999999.0	

NB: Approximate attenuation volume taken as 615m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 1.62Ha,</u> <u>Qmax= 28.2 litres/sec</u>

🗸 Quick Storage	Estimate		
	Variables		
Micro	FSR Rainfall V	Cv (Summer)	0.750
Drainage	Return Period (years) 100	Cv (Winter)	0.840
Variables	Region England and Wales V	Impermeable Area (ha)	1.620
Results	Map M5-60 (mm) 20.000	Maximum Allowable Discharge (I/s)	28.2
Design	Ratio R 0.300	Infiltration Coefficient (m/hr)	0.00000
Overview 2D		Safety Factor	2.0
		Climate Change (%)	40
Overview 3D			
Vt			
		Analyse OK	Cancel Help
	Enter Maximum Allowable Disc	harge between 0.0 and 999999.0	
🕖 Quick Storage	Estimate		- • •
	Results		
Micro	Global Variables require approximate s	torage	
Drainage	of between 801 m ³ and 1249 m ³ .		
Variables	These values are estimates only and s	hould not be used for design purpos	es.
Results			
Design			
Overview 2D			
Overview 3D			
Vt			
		Analyse OK	Cancel Help
	Enter Maximum Allowable Disc	harge between 0.0 and 999999.0	

NB: Approximate attenuation volume taken as 1249m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.

<u>4072 Parcel I VWH Land Partnership (Walshaw) Limited , Walshaw, Bury</u> <u>Quick Storage Estimate Calculation 03.01.2020</u>

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 1.58Ha, Qmax= 22.4 litres/sec

🕖 Quick Storage	Estimate		
	Variables		
Micro Drainage	FSR Rainfall V	Cv (Summer)	0.750
Diamage	Return Period (years) 30	Cv (Winter)	0.840
Variables	Region England and Wales 🗸	Impermeable Area (ha)	1.580
Results	Map M5-60 (mm) 20.000	Maximum Allowable Discharge (I/s)	22.4
Design	Ratio R 0.300	Infiltration Coefficient (m/hr)	0.00000
Overview 2D		Safety Factor	2.0
Overview 3D		Climate Change (%)	0
Vt			
VI			
		Analyse OK	Cancel Help
	Enter Climate Chan	ge between -100 and 600	
11			
🖉 Quick Storage			
	Results		
Quick Storage		storage	
Micro Drainage	Results		
Micro	Results Global Variables require approximate of between 361 m ³ and 600 m ³ .		
Micro Drainage	Results Global Variables require approximate of between 361 m ³ and 600 m ³ .		
Micro Drainage Variables	Results Global Variables require approximate of between 361 m ³ and 600 m ³ .		
Micro Drainage Variables Results	Results Global Variables require approximate of between 361 m ³ and 600 m ³ .		
Variables Results Design	Results Global Variables require approximate of between 361 m ³ and 600 m ³ .		
Variables Results Design Overview 2D	Results Global Variables require approximate of between 361 m ³ and 600 m ³ .		
Variables Variables Results Design Overview 2D Overview 3D	Results Global Variables require approximate of between 361 m ³ and 600 m ³ .	should not be used for design purpo	
Variables Variables Results Design Overview 2D Overview 3D	Results Global Variables require approximate of between 361 m ³ and 600 m ³ . These values are estimates only and	should not be used for design purpo	ses.

NB: Approximate attenuation volume taken as 600m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 1.58Ha,</u> <u>Qmax= 27.5 litres/sec</u>

💋 Quick Storage	Estimate		
	Variables		
Mitro	FSR Rainfall V	Cv (Summer)	0.750
Drainage	Return Period (years) 100	Cv (Winter)	0.840
Variables	Region England and Wales 🗸	Impermeable Area (ha)	1.580
Results	Map M5-60 (mm) 20.000	Maximum Allowable Discharge (1/s)	27.5
Design	Ratio R 0.300	Infiltration Coefficient (m/hr)	0.00000
Overview 2D		Safety Factor	2.0
Overview 3D		Climate Change (%)	40
Vt			
vi			
		Analyse OK	Cancel Help
	Enter Maximum Allowable Dis	charge between 0.0 and 999999.0	
7	-		
/ Quick Storage			
here.	Results		
Micro Drainage	Global Variables require approximate of between 781 m ³ and 1218 m ³ .	storage	
	These values are estimates only and	should not be used for design purp	oses.
Variables			
Results			
Design			
Overview 2D			
Overview 3D			
Vt			
		Analyse OK	Cancel Help
	Enter Maximum Allowable Dis	scharge between 0.0 and 999999.0	

NB: Approximate attenuation volume taken as 1218m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.

<u>4072 Spine Road 1 VWH Land Partnership (Walshaw) Limited , Walshaw, Bury</u> <u>Quick Storage Estimate Calculation 03.01.2020</u>

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 0.47Ha, Qmax= 6.7 litres/sec

Quick Storage	I see a second se					
	Variables					
Micro Drainage	FSR Rainfall	~	Cv (Summer)		0.750	
sianage	Return Period (years)	30	Cv (Winter)		0.840	
Variables	Region Englar	id and Wales 🔍 🗸	Impermeable Area (ha))	0.470	
Results	Map M5-60	(mm) 20.000	Maximum Allowable Di	scharge (l/s)	6.7	
Anne anne ann	Ratio F	0.300	Infiltration Coefficient (m∕hr)	0.00000	
Design	-	7	Safety Factor		2.0	
Overview 2D			Climate Change (%)		0	
Overview 3D						
Vt						
			Analyse	OK	Cancel	Help
Quick Storage	Estimate	Enter Climate Chan	ge between -100 and 600			
Quick Storage		Enter Climate Chan				
Micro	Results Global Variables re	quire approximate	ge between -100 and 600			
Micro	Results Global Variables re of between 107 m ³	quire approximate and 178 m ³ .	ge between -100 and 600 storage			
	Results Global Variables re of between 107 m ³	quire approximate and 178 m ³ .	ge between -100 and 600	r design purp		
Micro Drainage	Results Global Variables re of between 107 m ³	quire approximate and 178 m ³ .	ge between -100 and 600 storage	r design purp		
Micro Drainage Variables	Results Global Variables re of between 107 m ³	quire approximate and 178 m ³ .	ge between -100 and 600 storage	r design purp		
Variables Results Design	Results Global Variables re of between 107 m ³	quire approximate and 178 m ³ .	ge between -100 and 600 storage	r design purp		
Variables Results	Results Global Variables re of between 107 m ³	quire approximate and 178 m ³ .	ge between -100 and 600 storage	r design purp		
Variables Results Design Overview 2D	Results Global Variables re of between 107 m ³	quire approximate and 178 m ³ .	ge between -100 and 600 storage	r design purp		
Variables Variables Results Design Overview 2D Overview 3D	Results Global Variables re of between 107 m ³	quire approximate and 178 m ³ .	ge between -100 and 600 storage	r design purp		Help

NB: Approximate attenuation volume taken as 178m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 0.47Ha,</u> <u>Qmax= 8.2 litres/sec</u>

🗸 Quick Storage	Estimate		
	Variables		
Micro Drainage	FSR Rainfall 🗸	Cv (Summer)	0.750
Diamage	Return Period (years) 100	Cv (Winter)	0.840
Variables	Region England and Wales 🗸	Impermeable Area (ha)	0.470
Results	Map M5-60 (mm) 20.000	Maximum Allowable Discharge (I/s)	8.2
Design	Ratio R 0.300	Infiltration Coefficient (m/hr)	0.00000
		Safety Factor	2.0
Overview 2D		Climate Change (%)	40
Overview 3D			
Vt			
		Analyse OK	Cancel Help
	Enter Climate Chang	e between -100 and 600	
💋 Quick Storage	Estimate		
	Results		
Micro	Global Variables require approximate	storage	
Drainage	of between 232 m ³ and 362 m ³ .		
Variables	These values are estimates only and	should not be used for design purpo	ses.
Results			
Design			
Overview 2D			
Overview 3D			
Vt			
		Analyse OK	Cancel Help
	Enter Climate Chang	e between -100 and 600	

NB: Approximate attenuation volume taken as 362m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.

<u>4072 Spine Road 2 VWH Land Partnership (Walshaw) Limited , Walshaw, Bury</u> <u>Quick Storage Estimate Calculation 03.01.2020</u>

(MicroDrainage Source Control 2019.1)

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 0.26Ha,</u> <u>Qmax= 5.0 litres/sec</u>

1 Sec. 1	Variables			
Micro Drainage	FSR Rainfall	~	Cv (Summer)	0.750
Diamage	Return Period (years)	100	Cv (Winter)	0.840
Variables	Region Engla	and and Wales 🗸 🗸	Impermeable Area (ha)	0.260
Results	Map M5-60) (mm) 20.000	Maximum Allowable Discharge (1/s)	5.0
Design	Ratio	R 0.300	Infiltration Coefficient (m/hr)	0.00000
Overview 2D			Safety Factor	2.0
			Climate Change (%)	40
Overview 3D				
Vt				
			Analyse OK	Cancel Help
		Enter Area betwee	en 0.000 and 999.999	
Quick Storage	Estimate			
	Results			
	nesuis			
Micco	Global Variables n	equire anoroximate s	torace	
Micro Drainage	Global Variables n of between 124 m	equire approximate s ³ and 193 m ³ .	storage	
Drainage	of between 124 m	³ and 193 m ³ .	storage should not be used for design purpo	ses.
Micro Drainage Variables	of between 124 m	³ and 193 m ³ .		ses.
Drainage	of between 124 m	³ and 193 m ³ .		ses.
Drainage Variables	of between 124 m	³ and 193 m ³ .		ses.
Variables Results	of between 124 m	³ and 193 m ³ .		ses.
Variables Results Design	of between 124 m	³ and 193 m ³ .		ses.
Variables Results Design Overview 2D	of between 124 m	³ and 193 m ³ .		ses.
Variables Results Design Overview 2D Overview 3D	of between 124 m	³ and 193 m ³ .		ses.

Enter Area between 0.000 and 999.999

NB: Approximate attenuation volume taken as 193m³ subject to confirmation at detail design, maximum discharge restricted to minimum practical rates of 5 l/s in accordance with NPPF Guidelines.

<u>4072 Spine Road 3 VWH Land Partnership (Walshaw) Limited , Walshaw, Bury</u> <u>Quick Storage Estimate Calculation 03.01.2020</u>

(MicroDrainage Source Control 2019.1)

3.33% RP Event on Development Impermeable Area 0.63Ha, Qmax= 9.0 litres/sec

💋 Quick Storage	Estimate				
	Variables				
Micro Drainage	FSR Rainfall	~	Cv (Summer)	0.	750
Diamage	Return Period (years)	30	Cv (Winter)	0.	840
Variables	Region England and	i Wales 🗸 🗸	Impermeable Area (ha)	0.	630
Results	Map M5-60 (mm)	20.000	Maximum Allowable Disch	arge (1/s) 9.	0
Design	Ratio R	0.300	Infiltration Coefficient (m/h	ır) 0.	00000
Overview 2D			Safety Factor	2.	0
	-		Climate Change (%)	0	
Overview 3D	-				
Vt					
			Analyse	OK Ca	ancel Help
	Enter Maxim	um Allowable Disc	harge between 0.0 and 9999	0.00	
			haige between 0.0 and 5005	55.0	
💋 Quick Storage	Estimate				
	Results				
Micro Drainage	Global Variables require of between 143 m ³ and	approximate s 239 m ³ .	torage		
	These values are estimated	ates only and s	hould not be used for de	esign purposes.	
Variables					
Results					
Design					
Overview 2D					
Overview 3D					
Vt			HI		
			Analyse	OK Ca	Incel Help
	Enter Maximu	m Allowable Disc	harge between 0.0 and 9999	99.0	

NB: Approximate attenuation volume taken as 239m³ upper limit.

<u>1.00% RP Storm + 40% Climate change Event on Development Impermeable Area 0.63Ha,</u> <u>Qmax= 11.0 litres/sec</u>

🕖 Quick Storag	e Estimate				
	Variables				
Micro	FSR Rainfall		~	Cv (Summer)	0.750
Drainage	Return Period	d (years)	100	Cv (Winter)	0.840
Variables	Region	England and	Wales 🗸	Impermeable Area (ha)	0.630
Results	Map	M5-60 (mm)	20.000	Maximum Allowable Discharge (1/s)	11.0
Design		Ratio R	0.300	Infiltration Coefficient (m/hr)	0.00000
Overview 2D				Safety Factor	2.0
	_			Climate Change (%)	40
Overview 3D					
Vt					
				Analyse OK	Cancel Help
		East No. 1		barren barbarren 0.0 and 000000.0	
		Enter Maximu	im Allowable Disci	harge between 0.0 and 999999.0	
🕖 Quick Storag	e Estimate				
🖊 Quick Storag	e Estimate Results				
Micro	Results Global Varia	ables require	e approximate s	torage	
	Results Global Varia of between	311 m ³ and	485 m ³ .		
Micro	Results Global Varia of between	311 m ³ and	485 m ³ .	torage should not be used for design purp	
Micro Drainage	Results Global Varia of between	311 m ³ and	485 m ³ .		
Micro Drainage Variables	Results Global Varia of between	311 m ³ and	485 m ³ .		
Micro Drainage Variables Results	Results Global Varia of between	311 m ³ and	485 m ³ .		
Micro Drainage Variables Results Design	Results Global Varia of between	311 m ³ and	485 m ³ .		
Micro Drainage Variables Results Design Overview 2D	Results Global Varia of between	311 m ³ and	485 m ³ .		
Micro Drainage Variables Results Design Overview 2D Overview 3D	Results Global Varia of between	311 m ³ and	485 m ³ .		
Micro Drainage Variables Results Design Overview 2D Overview 3D	Results Global Varia of between	311 m ³ and	485 m³. ates only and s	should not be used for design purp	oses.

NB: Approximate attenuation volume taken as 485m³ upper limit, subject to further development at detail design stage when 100yr Storm 6hr duration pre/post-development volumes are checked.



COMPLEX CHALLENGES ... MADE SIMPLE

APPENDIX H – MASTERPLAN SURFACE WATER DRAINAGE STRATEGY PLANS









COMPLEX CHALLENGES ... MADE SIMPLE

APPENDIX I – MASTERPLAN FOUL WATER DRAINAGE STRATEGY PLANS

















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